DESIGN CRITERIA

1. CODES:
   INTERNATIONAL BUILDING CODE (IBC) 2009
   AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-08)
   AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-08)
   AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
   ALLOWABLE STRENGTH DESIGN (ASD)(AISC 360-05) THIRTEENTH EDITION, 2005
   AMERICAN WELDING SOCIETY D1.1
   AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS
   AMERICAN FOREST & PAPER ASSOCIATION (AF&PA) NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION, 2005

2. DESIGN LOADS:

OCCUPANCY CATEGORY	II
BACKFILL	
EQUIVALENT FLUID PRESSURE	70 PCF
SEISMIC (IBC)	
SOIL CLASSIFICATION	D
SPECTRAL RESPONSE ACCELERATION, Ss	0.105 g
SPECTRAL RESPONSE ACCELERATION, S1	0.044 g
SHORT PERIOD DESIGN ACCELERATION, Sds	0.112g
LONG PERIOD DESIGN ACCELERATION, Sd1	0.070 g
IMPORTANCE FACTOR	1.0
SEISMIC DESIGN CATEGORY	B
SEISMIC FORCE RESISTING SYSTEM	ORDINARY REINFORCED MASONRY SHEAR WALLS
RESPONSE MODIFICATION FACTOR, R	2
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC RESPONSE COEFFICIENT, Cs	0.056
DESIGN BASE SHEAR, V = Cs x W	
WIND - PARAMETERS	
BASIC WIND SPEED	90 MPH
IMPORTANCE FACTOR	1.0
EXPOSURE CLASS	B
WIND - MAIN WIND FORCE RESISTING SYSTEM PRESSURES	
DESIGN PRESSURE	15 PSF
ROOF UPLIFT PRESSURE	15 PSF (GROSS) [LC: 1.0WL]
ROOF UPLIFT PRESSURE	5 PSF (NET) [LC: 0.6DL + 1.0 WL]
WIND - ELEMENTS AND COMPONENTS	
PER APPLICABLE BUILDING CODE	
LIVE LOADS	
CORRIDOR AND PUBLIC SPACE	100 PSF UNREDUCIBLE
MECHANICAL	125 PSF UNREDUCIBLE
OFFICE	75 PSF REDUCIBLE
PARTITIONS	20 PSF UNREDUCIBLE
STAIRS	100 PSF UNREDUCIBLE
PHOTOVOLTAIC PANELS	10 PSF UNREDUCIBLE
GREEN ROOF SEDUM TRAYS	40 PSF UNREDUCIBLE
SNOW LOADS	
GROUND SNOW LOAD	30 PSF
SNOW EXPOSURE FACTOR	1.0
THERMAL FACTOR	1.0
IMPORTANCE FACTOR	1.0
FLAT-ROOF SNOW LOAD	21 PSF
DESIGN LOAD	25 PSF
DRIFTING LOAD	REFER TO PLAN

3. NET ALLOWABLE SOIL BEARING PRESSURES
   SPREAD FOOTINGS — 4000 PSF (PRESUMED)
   CONTINUOUS FOOTINGS — 4000 PSF (PRESUMED)
4. MINIMUM FROST PROTECTION DEPTH FROM ADJACENT GRADE:
   EXTERIOR FOOTING ADJACENT TO HEATED AREA — -4'-0"
   EXTERIOR FOOTINGS IN UNHEATED AREA — -4'-0"
5. SPECIFIED 28-DAY CONCRETE COMPRESSIVE STRENGTHS (f'c)
   ELEVATED SLABS — 4000 PSI
   FOOTINGS — 3000 PSI
   FOUNDATION WALLS — 4000 PSI
   SLABS ON GRADE — 4000 PSI
   TYPICAL - UNLESS NOTED OTHERWISE — 4000 PSI
6. CONCRETE REINFORCING STEEL SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO THE FOLLOWING STANDARDS:

DEFORMED BARS	ASTM A615, GRADE 60	Fy = 60 KSI
WELDED WIRE REINFORCING	ASTM A185	Fy = 65 KSI

7. MATERIALS FOR CONCRETE UNIT MASONRY SHALL CONFORM TO THE FOLLOWING STANDARDS:

CONCRETE MASONRY UNITS	ASTM C90
MORTAR MATERIALS	ASTM C270, TYPE S
GROUT FOR MASONRY	ASTM C476
REINFORCING STEEL FOR MASONRY	ASTM A615, GRADE 60 (UNO)
PLATE AND BENT BAR ANCHORS	ASTM A36
SHEET METAL ANCHORS AND TIES	ASTM A1008
WIRE MESH TIES	ASTM A185
WIRE TIES AND ANCHORS	ASTM A951
ANCHOR BOLTS FOR MASONRY	ASTM A307, GRADE A

8. MINIMUM 28 DAY COMPRESSIVE STRENGTHS FOR MASONRY (f'm):

DESIGN ASSEMBLY STRENGTH, f'm	2500 PSI
INDIVIDUAL CONCRETE MASONRY UNITS	3750 PSI
MORTAR FOR MASONRY (TYPE S REQUIRED)	1800 PSI
GROUT FOR MASONRY	2500 PSI

9. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

WIDE FLANGE SECTIONS	ASTM A992	Fy = 50 KSI
OTHER ROLLED SECTIONS	ASTM A36	Fy = 36 KSI
SQUARE AND RECTANGULAR HSS	ASTM A500, GR B	Fy = 46 KSI
PIPE SECTIONS	ASTM A53, GR B	Fy = 35 KSI
CAP AND BASE PLATES	ASTM A36	Fy = 36 KSI
CONNECTION MATERIAL	ASTM A36	Fy = 36 KSI
STIFFENER PLATES	ASTM A36	Fy = 36 KSI
ANCHOR RODS	ASTM F1554, GR 36	Fy = 36 KSI
HIGH STRENGTH BOLTS	A325 (3/4" DIAMETER UNO)	Fv = 24 KSI
TWIST-OFF BOLT/NUT/WASHER ASSEMBLIES	ASTM F1852	
HEAVY HEX NUTS	ASTM A563	
WASHERS	ASTM F436	
HEADED WELDED STEEL STUDS	ASTM A108, TYPE B	
ELECTRODES FOR ARC WELDING	AWS 5.1, E70XX	

10. COLD-FORMED STRUCTURAL STUDS SHALL CONFORM TO THE FOLLOWING STANDARDS:
    ROLLED SECTIONS, CONNECTION MATERIAL, STIFFENER PLATES

18 GAUGE AND THINNER	ASTM A653, GR 33	Fy = 33 KSI
16 GAUGE AND THICKER	ASTM A653, GR 50	Fy = 50 KSI
CONNECTION MATERIAL (>3/16" THICK)	ASTM A36	Fy = 36 KSI
ANCHOR RODS	ASTM F1554, GR 36	Fy = 36 KSI
BOLTS	ASTM A307	Fv = 10 KSI
COATING - HOT DIPPED	ASTM A924, G60	
ELECTRO - PLATE	ASTM A591	
ALUMINUM - ZINC	ASTM A792, GR 40	
INSTALLATION	ASTM C955 AND ASTM C1007	
ELECTRODES FOR ARC WELDING	AWS 5.1, E60XX	

11. STEEL DECK AND ALL ACCESSORIES SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO THE FOLLOWING STANDARDS:

GALVANIZED STEEL FLOOR DECK	ASTM A653, GR 50	Fy = 50 KSI
GALVANIZED STEEL ROOF DECK	ASTM A653, GR 33	Fy = 33 KSI

12. WOOD FRAMING:

HEADERS/RAFTERS/JOIST:	Fb = 850 PSI	Fc II = 1400 PSI
DOUGLAS FIR - LARCH;	NO. 2	
HEM FIR;	NO. 2	
SOUTHERN PINE;	NO. 2	

GENERAL NOTES

1. NEITHER THE PROFESSIONAL ACTIVITIES OF THE ENGINEER, NOR THE PRESENCE OF THE ENGINEER OR HIS OR HER EMPLOYEES AND SUBCONSULTANTS AT THE CONSTRUCTION SITE, SHALL RELIEVE THE CONTRACTOR AND ANY OTHER ENTITY OF THEIR OBLIGATIONS, DUTIES, AND RESPONSIBILITIES INCLUDING, BUT NOT LIMITED TO, CONSTRUCTION MEANS, METHODS, SEQUENCE, TECHNIQUES, OR PROCEDURES NECESSARY FOR PERFORMING, SUPERINTENDING, OR COORDINATING ALL PORTIONS OF THE WORK OF CONSTRUCTION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND ANY HEALTH OR SAFETY PRECAUTIONS REQUIRED BY ANY REGULATORY AGENCIES. THE ENGINEER AND HIS OR HER PERSONNEL HAVE NO AUTHORITY TO EXERCISE ANY CONTROL OVER ANY CONSTRUCTION CONTRACTOR OR OTHER ENTITY OR THEIR EMPLOYEES IN CONNECTION WITH THEIR WORK OR ANY HEALTH OR SAFETY PRECAUTIONS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE JOBSITE SAFETY. THE ENGINEER AND THE ENGINEER'S CONSULTANTS SHALL BE MADE ADDITIONAL INSUREDS UNDER THE CONTRACTOR'S GENERAL LIABILITY INSURANCE POLICY.
2. STRUCTURAL DRAWINGS INCLUDE DESIGN REQUIREMENTS AND DIMENSIONS FOR STRUCTURAL INTEGRITY BUT DO NOT SHOW ALL DETAIL DIMENSIONS TO FIT INTRICATE ARCHITECTURAL AND MECHANICAL DETAILS. CONTRACTOR SHALL SO CONSTRUCT THE WORK SO THAT IT WILL CONFORM TO THE CLEARANCES REQUIRED BY ARCHITECTURAL, MECHANICAL AND ELECTRICAL DESIGN.
3. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS NOTED OTHERWISE, THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION.
4. DETAILS AND NOTES ON THE STRUCTURAL DRAWINGS ARE INTENDED TO BE TYPICAL FOR SIMILAR SITUATIONS ELSEWHERE.
5. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL, AND PLUMBING WITH APPROPRIATE TRADE CONTRACTORS. OPENING SIZES AND LOCATIONS SHOWN FOR DUCTS, PIPES, INSERTS AND OTHER PENETRATIONS WHEN SHOWN ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED PRIOR TO FORMING.
6. DIMENSIONS, NOTES, AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
7. WHERE NEW CONSTRUCTION INTERFACES WITH EXISTING CONDITIONS, FIELD VERIFY EXISTING DIMENSIONS, MEMBER SIZES AND ELEVATIONS SHOWN ON THE DRAWINGS PRIOR TO STARTING CONSTRUCTION. ALL DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT.
8. REFER TO ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
   A. SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, UNLESS NOTED OTHERWISE.
   B. SIZE AND LOCATIONS OF ALL INTERIOR AND EXTERIOR MASONRY WALLS.
   C. SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGES IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC.
   D. SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS UNLESS NOTED OTHERWISE.
   E. FLOOR, WALL AND ROOF FINISHES.
   F. STAIR FRAMING AND DETAILS. ALSO REFER TO STAIR MANUFACTURER'S APPROVED SHOP DRAWINGS
   G. DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
   H. FIRE PROTECTION REQUIREMENTS.
9. REFER TO MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:
   A. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN.
   B. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
   C. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.
   D. SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES OR CURBS AND ANCHOR BOLTS FOR MOTOR MOUNTS.
10. BEFORE SUBMITTING A PROPOSAL FOR THIS WORK, EACH BIDDER SHALL VISIT THE PREMISES AND BECOME FULLY ACQUAINTED WITH THE EXISTING CONDITIONS, TEMPORARY CONSTRUCTION REQUIRED, QUANTITIES AND TYPES OF EQUIPMENT, ETC. THE BID SHALL INCLUDE ALL SUMS REQUIRED TO DO THE WORK WITHIN THE EXISTING CONDITIONS. DISRUPTION OF NORMAL ACTIVITIES IN THE WORK AREA SHALL BE KEPT TO A MINIMUM.
11. SHOP DRAWINGS PREPARED BY SUPPLIERS, SUBCONTRACTORS, AND OTHERS SHALL BE REVIEWED AND COORDINATED PRIOR TO SUBMITTING TO THE ARCHITECT. EACH SHOP DRAWING SUBMITTED SHALL BE STAMPED, INITIALED AND DATED INDICATING REVIEW BY THE CONSTRUCTION MANAGER/GENERAL CONTRACTOR.
12. SHOP DRAWINGS PREPARED BY THE SUBCONTRACTORS, SUPPLIERS, AND OTHERS SHALL BE REVIEWED BY THE ARCHITECT ONLY FOR GENERAL CONFORMANCE WITH DESIGN CONCEPT ONLY. REVIEW BY THE ARCHITECT SHALL NOT BEGIN WITHOUT THE PRIOR COORDINATION AND REVIEW BY THE GENERAL CONTRACTOR. WORK SHALL NOT BEGIN WITHOUT REVIEW BY THE ARCHITECT. NOTATIONS MADE BY THE ARCHITECT ON THE SHOP DRAWINGS DO NOT RELIEVE THE CONTRACTOR FROM COMPLYING WITH THE REQUIREMENTS OF THE DRAWINGS AND SPECIFICATIONS.
13. OPTIONS ARE FOR THE CONTRACTOR'S CONVENIENCE. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES RESULTING FROM CHOOSING AN OPTION AND SHALL COORDINATE ALL DETAILS. THE COST OF ADDITIONAL DESIGN WORK NECESSITATED BY SELECTION OF AN OPTION SHALL BE BORNE BY THE CONTRACTOR.
14. THE COST OF ADDITIONAL DESIGN WORK DUE TO ERRORS OR OMISSIONS BY THE CONTRACTOR IN CONSTRUCTION SHALL BE BORNE BY THE CONTRACTOR.
15. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW OR RECORD SHALL BEAR THE STAMP AND SIGNATURE OF A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE OF WISCONSIN.
16. ELEVATIONS ARE BASED ON THE EXISTING FIRST FLOOR ELEVATION OF (+ 100' - 0") WHICH IS EQUAL TO CIVIL ELEVATION OF (913.5').

FOUNDATIONS/SLAB-ON-GRADE

1. CROSS REFERENCE ARCHITECTURAL AND STRUCTURAL DRAWINGS TO ASSURE PROPER DIMENSIONS AND PLACEMENT OF ALL ANCHOR BOLTS, INSERTS, NOTCHES, EDGES IN GRADE BEAMS, FOUNDATION WALLS AND PIERS.
2. FOUNDATION DESIGN BASED ON INFORMATION FROM THE EXISTING 1926 DRAWINGS. SOIL BEARING CAPACITY TO BE FIELD VERIFIED PRIOR TO POURING NEW FOUNDATIONS. NOTIFY A/E IF EXISTING CONDITIONS DIFFER FROM THOSE INDICATED.
3. ALL EXCAVATIONS SHALL BE PROPERLY AND SAFELY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE HAS ATTAINED SPECIFIED COMPRESSIVE STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL WALLS BELOW GRADE FROM LATERAL LOADS UNTIL SUPPORTING FLOOR IS COMPLETELY IN PLACE AND HAS ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS, AND INSTALLATION OF SHORING AND/OR SHEETING. BACKFILLING IS NOT PERMITTED FOR FOUNDATION WALLS UNTIL SUPPORTED SLAB ABOVE IS IN PLACE OR THE WALL IS ADEQUATELY BRACED TO RESIST LATERAL LOADS.
4. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL BE CENTERED UNDER WALLS, PIERS OR COLUMNS.
5. PROVIDE SAW CUT CONTROL JOINTS IN ALL SLABS-ON-GRADE. LOCATE JOINTS ALONG COLUMN LINES WITH INTERMEDIATE JOINTS SPACED AT A MAXIMUM OF 36 TIMES THE SLAB THICKNESS, UNLESS NOTED OTHERWISE. CONTROL JOINTS SHALL BE CONTINUOUS, NOT STAGGERED OR OFFSET. SLAB PANELS SHALL HAVE A MAXIMUM LENGTH TO WIDTH RATIO OF 1.5 TO 1. PROVIDE ADDITIONAL CONTROL JOINTS AT ALL RE-ENTRANT CORNERS FORMED IN SLAB ON GRADE.

UNDERPINNING NOTES

1. UNDERPIN EXISTING WALLS AND FOOTINGS AND PLACE NEW FOOTINGS AS SHOWN ON THE FOUNDATION PLAN AND DETAILS.
2. NO EXCAVATION CAN BE DONE WITHIN 6'-0" OF THE EXISTING BUILDING UNTIL UNDERPINNING IS COMPLETE.
3. POURED CONCRETE WALL UNDERPINNING PROCEDURE SHALL BE AS FOLLOWS:
   A. WHERE NOTED ON THE PLANS AND DETAILS, UNDERPINNING SHALL BE PERFORMED IN SEPARATE OPERATIONS NOT TO EXCEED 6'-0" IN LENGTH.
   B. PLACE HORIZONTAL REINFORCING BARS WITH 90-DEGREE BENDS SO THAT THEY MAY BE STRAIGHTENED TO LAP WITH BARS OF ADJACENT UNDERPINNING SECTIONS.
   C. PLACE CONCRETE OF NEW WALL TO WITHIN 3"-4" OF THE UNDERSIDE OF EXISTING FOOTING WHERE APPLICABLE. AFTER 2 DAYS PACK FINAL 3"-4" WITH DRY GROUT. USE SAME GROUT SPECIFIED UNDER STEEL BASE PLATES GIVEN IN SPECIFICATION SECTION 05120. GROUT IS TO SET AT LEAST 7 DAYS BEFORE ADJACENT SECTION IS EXCAVATED. SHORE AND BRACE EACH SECTION AS SOON AS FORMS ARE REMOVED. WHEN EXCAVATION IS STARTED IN ANY SECTION, ALL WORK IN THAT SECTION MUST BE COMPLETED PROMPTLY AND PRIOR TO STARTING ANOTHER SECTION IN THE IMMEDIATE AREA.
   D. UNLESS NOTED OTHERWISE, NEW FOUNDATION WALL AND FOOTING TO MATCH EXISTING WALL THICKNESS AND FOOTING SIZE.
   E. PRIOR TO BACKFILLING AGAINST THE FOUNDATION WALL, SLAB-ON-GRADE MUST BE INSTALLED.
   F. REPEAT THIS PROCEDURE IN ALTERNATE 6'-0" SECTIONS UNTIL UNDERPINNING IS COMPLETE.
4. DURING UNDERPINNING OPERATION, CONTRACTOR SHALL BRACE AND SHORE WITH JACKS EXISTING FOOTING OR WALL AS REQUIRED TO PREVENT MOVEMENT OF EXISTING BUILDING. THE SHORING SHALL BE SELECTED TO WITHSTAND THE DESIGN FORCES INDICATED ON THE DRAWINGS. THE SHORING JACKS ARE TO BE LEFT IN PLACE AND CAST INTO THE CONCRETE UNDERPINNING POUR.
5. CONTRACTOR SHALL SUBMIT A SCHEDULE OF UNDERPINNING OPERATIONS FOR REVIEW OF THE ARCHITECT AND ENGINEER.
6. CONTRACTOR SHALL SURVEY EXISTING FOOTING AND WALL ELEVATIONS PRIOR TO, DURING, AND AFTER UNDERPINNING OPERATIONS TO MONITOR POTENTIAL MOVEMENTS. SURVEY RESULTS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER.

REINFORCING STEEL

1. FOR CAST-IN-PLACE CONCRETE THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER	
NO. 6 BARS OR LARGER	2 INCHES
NO. 5 BARS OR SMALLER	1 1/2 INCHES
SLABS, WALLS, JOISTS NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	
NO. 14 AND NO. 18 BARS	1 1/2 INCHES
NO. 11 BARS OR SMALLER	3/4 INCHES
BEAMS AND COLUMNS NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	1 1/2 INCHES

2. DIMENSIONS OF CONCRETE COVER FOR REINFORCEMENT INDICATED ON DRAWINGS ARE TO OUTERMOST REINFORCING BARS. FOR BEAMS OR COLUMNS WITH STIRRUPS OR TIES, CLEAR COVER INDICATED IS TO STIRRUPS OR TIES.
3. BAR SPLICES: SPLICE REINFORCING WHERE INDICATED ON THE DRAWINGS. ALL SPLICES SHALL BE CLASS 'B' AS DEFINED IN ACI 318. IF SPLICE LENGTH IS NOT GIVEN ON THE DRAWINGS, PROVIDE LAP LENGTHS (IN INCHES) AS FOLLOWS:

	3000 PSI CONCRETE		4000 PSI CONCRETE	
BAR SIZE	OTHER	TOP	OTHER	TOP
#3	22	28	19	25
#4	29	38	25	33
#5	36	47	31	41
#6	43	56	37	49
#7	63	81	54	71
#8	72	93	62	81
#9	81	105	70	91
#10	91	118	79	102
#11	101	131	87	114

LAP LENGTHS ASSUME CLEAR SPACING BETWEEN BARS OF 2 BAR DIAMETERS, AND A MINIMUM COVER OF 1 BAR DIAMETER. FOR DEVELOPMENT LENGTHS, DIVIDE BY 1.3. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 1'-0" OF FRESH CONCRETE BELOW.

4. ADHESIVE FOR DOWELING SHALL BE HILTI HIT HY 200, POWERS PE 1000+, OR SIMPSON SET XP. EMBEDMENT LENGTH SHALL BE AS INDICATED ON THE DRAWINGS. INSTALL PER MANUFACTURER'S INSTALLATION INSTRUCTIONS.

MASONRY (CONCRETE MASONRY UNITS)

1. MORTAR SHALL CONFORM TO AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) DESIGNATION CITED UNDER DESIGN CRITERIA, AND PROJECT SPECIFICATIONS. REFER TO DESIGN CRITERIA FOR MINIMUM COMPRESSIVE STRENGTH OF MORTAR.
2. REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE AND HEIGHT OF UNITS, LAYING PATTERN AND JOINT TYPE. ALL BLOCK SHALL BE RUNNING BOND UNLESS NOTED OTHERWISE.
3. THE LOAD BEARING CONCRETE MASONRY WALLS FOR THIS PROJECT WERE DESIGNED TO SPAN VERTICALLY AND BE BRACED BY THE ROOF AND FLOOR FRAMING ELEMENTS OF THE STRUCTURE. DURING CONSTRUCTION THE MASONRY CONTRACTOR SHALL PROVIDE LATERAL BRACING UNTIL THE ROOF STRUCTURE IS INSTALLED AS RECOMMENDED BY ACI 530 AND THE LATEST REVISION OF "STANDARD PRACTICE FOR BRACING MASONRY WALLS UNDER CONSTRUCTION", PREPARED BY THE COUNCIL FOR MASONRY WALL BRACING. THIS BRACING IS TO PREVENT UNNECESSARY STRESS OR DAMAGE TO THE MASONRY WALLS FROM WIND LOADS, WHICH CAN OCCUR WHILE THE WALLS ARE NOT PROPERLY BRACED BY THE ROOF AND FLOOR STRUCTURE.
4. BAR SPLICES: SPLICE REINFORCING WHERE INDICATED ON THE DRAWINGS. IF SPLICE LENGTH IS NOT GIVEN ON THE DRAWINGS PROVIDE LAP LENGTHS (IN INCHES) AS FOLLOWS. BARS LARGER THAN #9 ARE TO BE MECHANICALLY SPLICED.

MINIMUM LAP SPLICE LENGTH	
BAR SIZE	LAP LENGTH
#3	27
#4	36
#5	45
#6	54

POST INSTALLED STEEL ANCHORS

1. POST INSTALLED EXPANSION ANCHORS SERVING AS THE BASIS OF DESIGN ARE SHOWN ON THE DRAWINGS. ACCEPTABLE ALTERNATE ANCHORS MAY BE SUPPLIED PROVIDED THAT THE QUANTITY AND CONFIGURATION MATCHES THE CAPACITY OF THE DESIGN ANCHOR QUANTITY AND CONFIGURATION. ANY ACCEPTABLE ALTERNATES ARE TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. INSTALL IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. THE FOLLOWING TABLE SUMMARIZES THE EXPANSION ANCHORS USED ON THE PROJECT:

ANCHORED INTO:	BASIS OF DESIGN	ACCEPTABLE ALTERNATES AT CONTRACTOR'S OPTION
HOLLOW CMU	HILTI HLC SLEEVE	POWERS LOK/BOLT, ITW/RED HEAD DYNABOLT SLEEVE
GROUTED CMU	HILTI KWIK BOLT 3	POWER STUD+ SD1, SIMPSON WEDGE-ALL
UNCRACKED CONCRETE	HILTI KWIK BOLT 3	POWER STUD+ SD2, ITW/RED HEAD TRUBOLT+, SIMPSON STRONG BOLT
CRACKED CONCRETE	HILTI KWIK BOLT TZ	POWER STUD+ SD2, ITW/RED HEAD TRUBOLT+, SIMPSON STRONG BOLT

2. ADHESIVE ANCHOR SYSTEMS FOR ATTACHMENT INTO CONCRETE SHALL CONSIST OF ASTM A193 GRADE B7 RODS, HEAVY DUTY NUTS AND WASHERS, AND A TWO COMPONENT STRUCTURAL ADHESIVE. ADHESIVE ANCHORING SYSTEMS SERVING AS THE BASIS OF DESIGN ARE SHOWN ON THE DRAWINGS. ACCEPTABLE ALTERNATE ANCHORS MAY BE SUPPLIED PROVIDED THAT THE QUANTITY AND CONFIGURATION MATCHES THE CAPACITY OF THE DESIGN ANCHOR QUANTITY AND CONFIGURATION. ANY ACCEPTABLE ALTERNATES ARE TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. INSTALL IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. ANCHORING SYSTEMS INTO HOLLOW CMU SHALL INCLUDE A SCREEN TUBE. THE FOLLOWING TABLE SUMMARIZES THE ADHESIVE ANCHORS USED ON THE PROJECT:

ANCHORED INTO:	BASIS OF DESIGN	ACCEPTABLE ALTERNATES AT CONTRACTOR'S OPTION
HOLLOW CMU	HILTI HIT HY 70	POWERS AC 100+ GOLD, ITW A7 ACRYLIC
GROUTED CMU	HILTI HIT HY 70	POWERS AC 100+ GOLD, ITW A7 ACRYLIC, SIMPSON SET
CRACKED/UNCRACKED CONCRETE	HILTI HIT HY 200	POWERS PE 1000+, SIMPSON SET XP

STRUCTURAL STEEL

1. REFER TO DRAWINGS FOR DETAIL OF DECK OPENINGS. REFER TO ARCHITECTURAL MECHANICAL, ELECTRICAL DRAWINGS, ETC., FOR EXACT SIZE, LOCATION, AND COUNT OF REQUIRED OPENINGS.
2. UNLESS NOTED OTHERWISE ALL WELDS SHALL BE CONTINUOUS 1/4" FILLET WELDS.
3. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS." SEE DESIGN CRITERIA FOR BOLT SIZE AND MATERIAL ASTM DESIGNATION.
4. BOLTS IN SLOTTED HOLES SHALL BE LOCATED IN THE CENTER OF THE HOLE AFTER FIELD ASSEMBLY IS COMPLETE, UNLESS DETAILED OTHERWISE.

STEEL DECK

1. DECK SIZE AND GAGE INDICATED IN THE DRAWINGS ARE BASED ON THE FOLLOWING:
   A. VULCRAFT 2008 CATALOG FOR GRAVITY DESIGN LOADS.
   B. STEEL DECK INSTITUTE (SDI) DIAPHRAGM DESIGN MANUAL 3RD EDITION FOR DIAPHRAGM LOADS.
   C. VULCRAFT 2008 CATALOG FOR UNSHORED CONSTRUCTION SPANS.
2. STEEL ROOF DECK GALVANIZING SHALL CONFORM TO ASTM A924 WITH A MINIMUM COATING OF G60.
3. CORRUGATED STEEL FORM DECK GALVANIZING SHALL CONFORM TO ASTM A924 WITH A MINIMUM COATING OF G60.
4. UNLESS NOTED OTHERWISE, DECK SHALL BE FASTENED WITH 5/8" DIAMETER PUDDLE WELDS AT 12" OC AT ALL SUPPORTS AND EDGES. SIDE LAPS SHALL BE FASTENED WITH #10 TEK SCREWS, MINIMUM ONE AT EACH MIDSPAN. OPENING EDGES SHALL RECEIVE THE SAME WELDING AS REQUIRED AT DECK ENDS. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN COLD-FORMED STEEL DECK WORK.
5. DO NOT EXCEED 25 LBS PER HANGER AND A MINIMUM SPACING OF 2'-0" ON CENTER WHEN ATTACHING TO STEEL ROOF DECKING (LIMITATION NOT REQUIRED WITH CONCRETE ON STEEL DECK). THIS 25 LBS LOAD AND 2'-0" SPACING INCLUDES ADJACENT MECHANICAL, ELECTRICAL, AND ARCHITECTURAL ITEMS HANGING FROM DECK. IF THE HANGER RESTRICTIONS CANNOT BE ACHIEVED, SUPPLEMENTAL FRAMING SUPPORTED OFF STEEL FRAMING WILL NEED TO BE ADDED. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING LOCATION AND WEIGHT OF ALL THE ELEMENTS BEING HUNG.
6. USE SUMP PANS AT ALL ROOF DRAINS. MINIMUM THICKNESS FOR SUMP PANS SHALL BE 14 GAGE.

LINTELS

1. PROVIDE LINTELS OVER ALL OPENINGS AND RECESSES IN MASONRY CONSTRUCTION.
2. THE STRUCTURAL DOCUMENTS REFLECT THE BEST ATTEMPT TO IDENTIFY ALL WALL PENETRATIONS IN THE EXISTING AND NEW CONSTRUCTION. PENETRATIONS NOT IDENTIFIED ON THE DOCUMENTS ARE TO BE TREATED IN A MANNER SIMILAR TO THE IDENTIFIED LOCATIONS. LINTELS IN NON-BEARING MASONRY WALL OPENINGS CAN BE SIZED IN ACCORDANCE WITH THE NOTE BELOW. LINTELS THAT OCCUR IN EXISTING BEARING WALLS ARE TO BE SIZED ACCORDING TO SIMILAR CONDITIONS AND SPANS IN THE NEW CONSTRUCTION AND LINTEL SCHEDULE. BOTTOM PLATE SIZE SHALL BE A MINIMUM OF 3/8" THICK. THE WIDTH OF THE PLATE SHALL BE 3/4" LESS THAN THE FIELD VERIFIED WALL THICKNESS. THE PLATE SHALL BE THE FULL LENGTH OF THE LINTEL MEMBER. LINTELS ARE NOT REQUIRED OVER OPENINGS THAT ARE 12" WIDE OR LESS AND AT LEAST 1 COURSE BELOW THE TOP OF THE WALL.
3. ALL LINTELS SHALL HAVE A MINIMUM OF 8" END BEARING.
4. ALL LINTELS IN EXTERIOR WALL CONSTRUCTION SHALL BE HOT-DIP GALVANIZED, UNO.
5. FOR ALL OPENINGS NOT OTHERWISE DETAILED OR SCHEDULED, MINIMUM LINTELS SHALL BE FOR EACH 4 INCH OF MASONRY WIDTH:

0 TO 2'-0" SPAN	5/16" PLATE (3/4" LESS THAN WALL WIDTH)
2'-0" TO 4'-0" SPAN	L 3 1/2x3 1/2x1/4
4'-0" TO 6'-0" SPAN	L4x3 1/2x5/16 (LLV)
6'-0" TO 8'-0" SPAN	L5x3 1/2x5/16 (LLV)

ALL ANGLES THAT ARE BACK TO BACK SHALL BE WELDED TOP AND BOTTOM 3" AT 12" MINIMUM.

6. BEARING PLATES NOT REQUIRED FOR LINTELS UNLESS NOTED OTHERWISE.

STRUCTURAL COLD-FORMED STEEL FRAMING (CFSF)

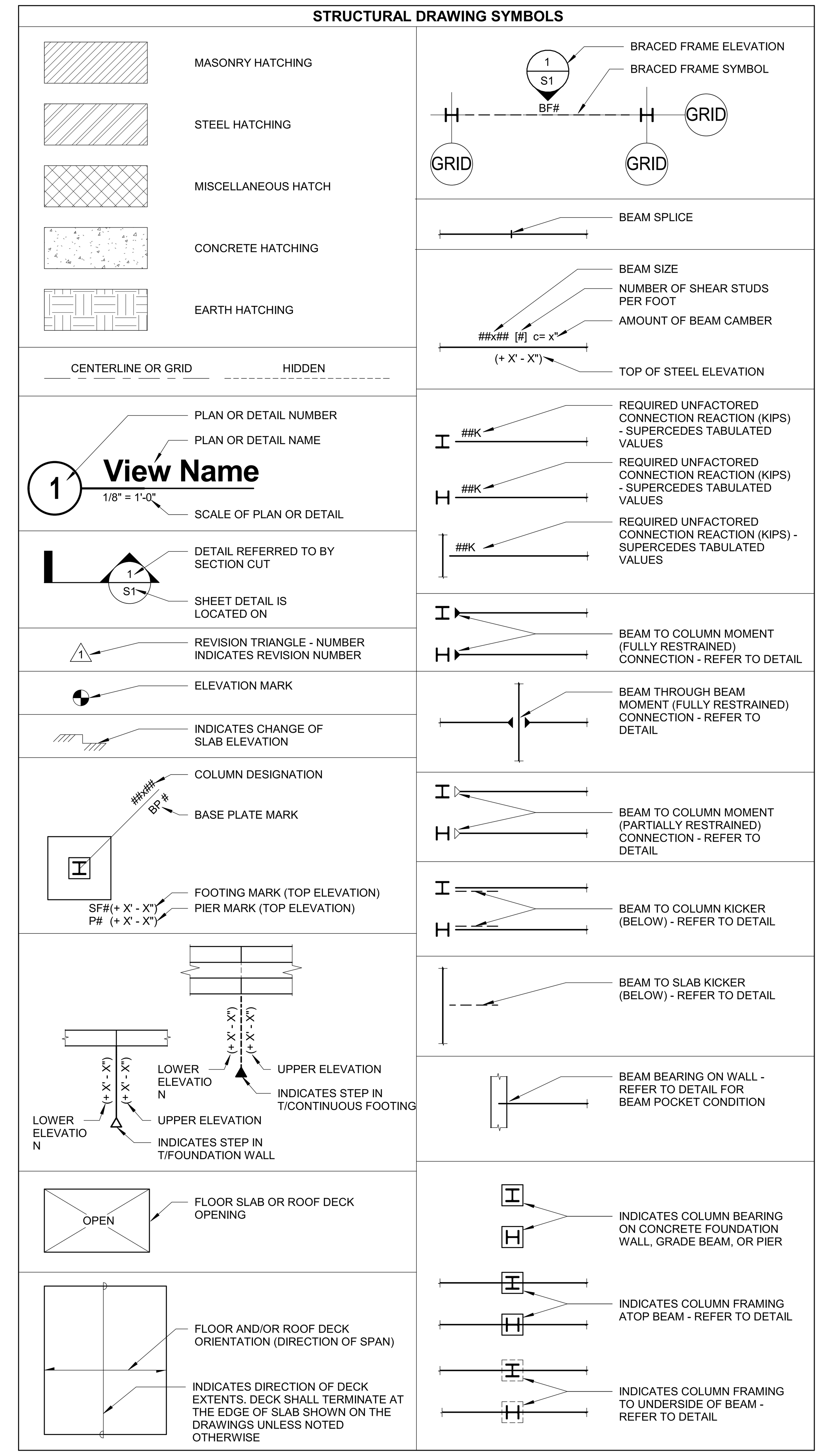
1. MATERIAL, DESIGN AND MANUFACTURE SHALL BE IN ACCORDANCE WITH THE "STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS" OF THE AMERICAN IRON AND STEEL INSTITUTE CURRENT EDITION.
2. STRUCTURAL COLD FORM STEEL FRAMING IS DEFINED AS THE FOLLOWING:
   A. ANY COLD FORMED FRAMING THICKER THAN 20 GA (33 MIL).
   B. ANY EXTERIOR COLD FORMED FRAMING.
   C. ALL OTHER STEEL STUD FRAMING IS NON-STRUCTURAL AND NOT A PART OF THE STRUCTURAL PACKAGE.
3. STRUCTURAL CFSF IS PERFORMANCE SPECIFIED. DESIGN INFORMATION INCLUDED IN THESE DOCUMENTS ARE TO BE CONSIDERED GUIDELINES FOR BIDDING PURPOSES ONLY. STUD DEPTH IS REQUIRED TO MEET THOSE INDICATED IN THE PLANS. CONNECTION DETAILS ARE ONLY AN INDICATION OF SUGGESTED SUPPORT AND SLIP JOINT ORIENTATION. GAUGE, SECTION, MATERIAL, BRACING, CONNECTIONS, STIFFENERS, AND SIMILAR DETAILS ARE THE RESPONSIBILITY OF THE MANUFACTURER BASED ON LOADS GIVEN ON THE PLANS AND SPECIFICATIONS.
4. CONSTRUCTION SHALL NOT BEGIN UNTIL SHOP DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD AND THE ARCHITECT.

EXISTING STRUCTURAL INFORMATION

1. EXISTING STRUCTURAL INFORMATION SHOWN WAS OBTAINED FROM EXISTING DRAWINGS DATED:
   A. 1926 BY THE US GOVERNMENT
   B. 1979 BY FLAD AND ASSOCIATES

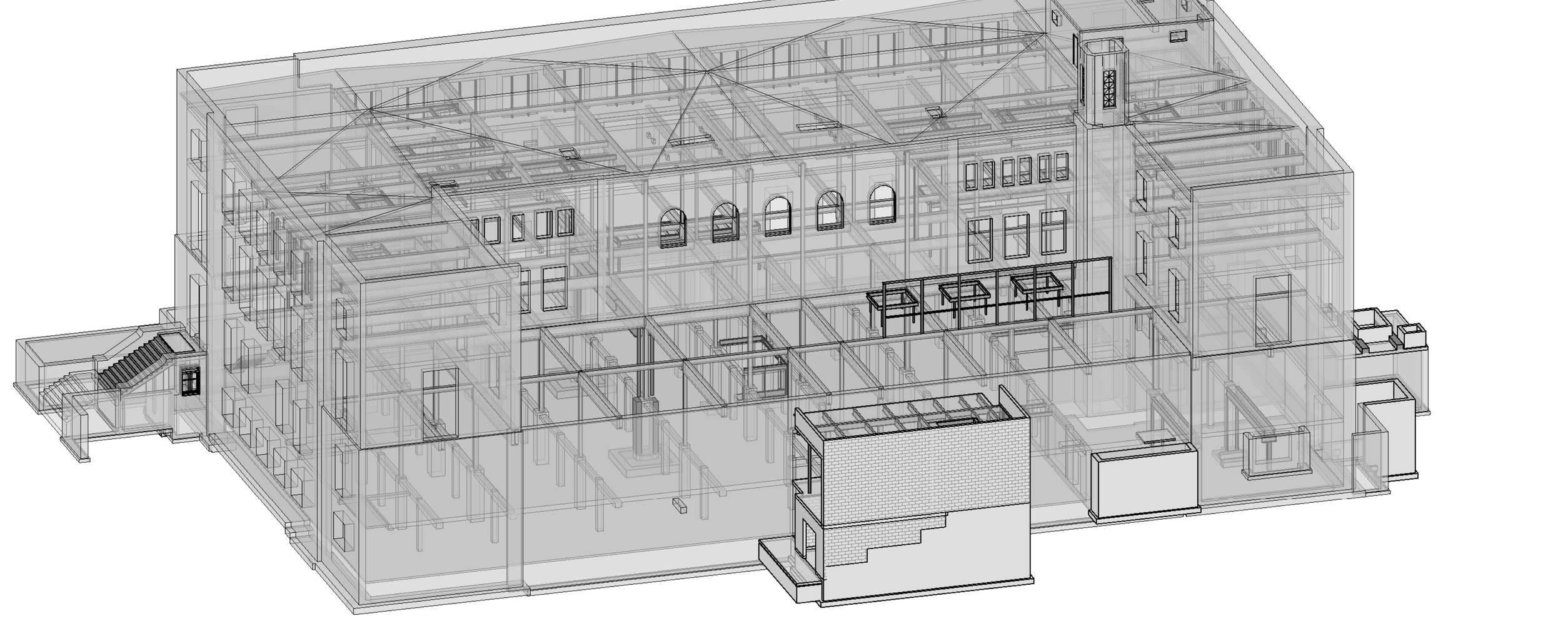
CONTRACTOR TO VERIFY EXISTING INFORMATION, DIMENSIONS, AND SIZES AS REQUIRED TO COMPLETE THEIR WORK.

STRUCTURAL DRAWING SYMBOLS



STRUCTURAL ABBREVIATIONS LIST

#	NUMBER	KSF	KIPS PER SQUARE FOOT
@	AT	L	LENGTH
°	DEGREES	LB	POUND
Ø	DIAMETER	LF	LINEAR FOOT
AHU	AIR-HANDLING UNIT	LL	LIVE LOAD
APPROX	APPROXIMATE, -LY	LLH	LONG LEG HORIZONTAL
ARCH	ARCHITECT, -URE, -URAL	LLV	LONG LEG VERTICAL
B/	BOTTOM OF	LSH	LONG SIDE HORIZONTAL
bf	BEAM FLANGE WIDTH	LSV	LONG SIDE VERTICAL
BM	BEAM	LONG	LONGITUDINAL
BP	BASE PLATE	M/E	MECHANICAL/ELECTRICAL
BRG	BEARING	MAX	MAXIMUM
CFSF	COLD FORM STEEL FRAMING	MECH	MECHANICAL
CJ	CONTROL JOINT	MEZZ	MEZZANINE
CL	CLEAR	MIN	MINIMUM
CMU	CONCRETE MASONRY UNIT	MISC	MISCELLANEOUS
CONC	CONCRETE	MK	MARK
CONST	CONSTRUCTION	N	NORTH
CONT	CONTINUOUS	N	LENGTH (AS PLATES)
D	DEPTH	NIC	NOT IN CONTRACT
DBL	DOUBLE	NO	NUMBER
DEG	DEGREE	NTS	NOT TO SCALE
DIM	DIMENSION	OC	ON CENTER
DL	DEAD LOAD	OPNG	OPENING
DTL	DETAIL	OPP	OPPOSITE
DWG	DRAWING	PAF	POWER ACTUATED FASTENER
EA	EACH	PC	PRECAST
EF	EACH FACE	PCF	POUNDS PER CUBIC FOOT
EJ	EXPANSION JOINT	PL	PLATE
EL	ELEVATION	PSF	POUNDS PER SQUARE FOOT
ELEC	ELECTRICAL	PSI	POUNDS PER SQUARE INCH
EMBED	EMBEDDED	PVC	POLYVINYL CHLORIDE
EOD	EDGE OF DECK	R	RADIUS
EOS	EDGE OF SLAB	RD	ROOF DRAIN
EQ	EQUAL	REINF	REINFORCING, -MENT, -ED
EQUIP	EQUIPMENT	REQ'D	REQUIRED
EW	EACH WAY	REF	REFERENCE, REFER TO
EXIST, (E)	EXISTING	RTU	ROOF-TOP UNIT
EXP	EXPANSION	SC	TC WITH CLASS A FAYING SURFACE
EXT	EXTERIOR	SCHED	SCHEDULE
f'c	CONCRETE COMPRESSIVE STRENGTH	SIM	SIMILAR
FDN	FOUNDATION	SL	SNOW LOAD
FIN	FINISHED	SP	SPACE(S)
FL	FLOOR	SPEC	SPECIFICATION(S)
FT	FOOT	SPEC'D	SPECIFIED
FTG	FOOTING	SQ	SQUARE
Fy	YIELD STRESS	STD	STANDARD
GA	GAGE OR GAUGE	STIFF	STIFFENER
GALV	GALVANIZED	T/	TOP OF
GB	GRADE BEAM	TC	PRE-TENSIONED BOLT
GC	GENERAL CONTRACTOR	TEMP	TEMPERATURE
GYP	GYPSUM	tf	BEAM FLANGE THICKNESS
HDG	HOT-DIPPED GALVANIZED	TRANS	TRANSVERSE
HORIZ	HORIZONTAL	TYP	TYPICAL
HVAC	HEATING, VENTILATION, AIR CONDITIONING	UNO	UNLESS NOTED OTHERWISE
HWS	HEADED, WELDED STUD	VERT	VERTICAL
IN	INCH	VIF	VERIFY IN FIELD
INT	INTERIOR	VWA	VERIFY WITH ARCHITECTURAL DRAWINGS
JST	JOIST	WP	WORKING POINT
JT	JOINT	WT	WEIGHT
K, KIP	KILOPOUND (1,000 POUNDS)	WWR	WELDED WIRE REINFORCING
KO	KNOCK-OUT	YD	YARD



3D VIEW

NOTES:

1. 3D VIEW IS FOR REFERENCE ONLY - NOT FOR CONSTRUCTION. REFER TO PLANS, DETAILS AND SPECIFICATIONS FOR ACTUAL CONSTRUCTION REQUIREMENTS.

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I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the Laws of the State of Wisconsin.
ARCHITECT SEAL

Signature: ____
Print Names: ____
Date: ____ License No: ____

ISSUE

MARK	DATE	DESCRIPTION
	03.24.2017	BID SET

PROJECT NO. 2014057

PROJECT PHASE: BID SET

DRAWN BY: PRIPAN — CHECKED BY: ABBPER

GENERAL NOTES

EXHIBIT H

S000

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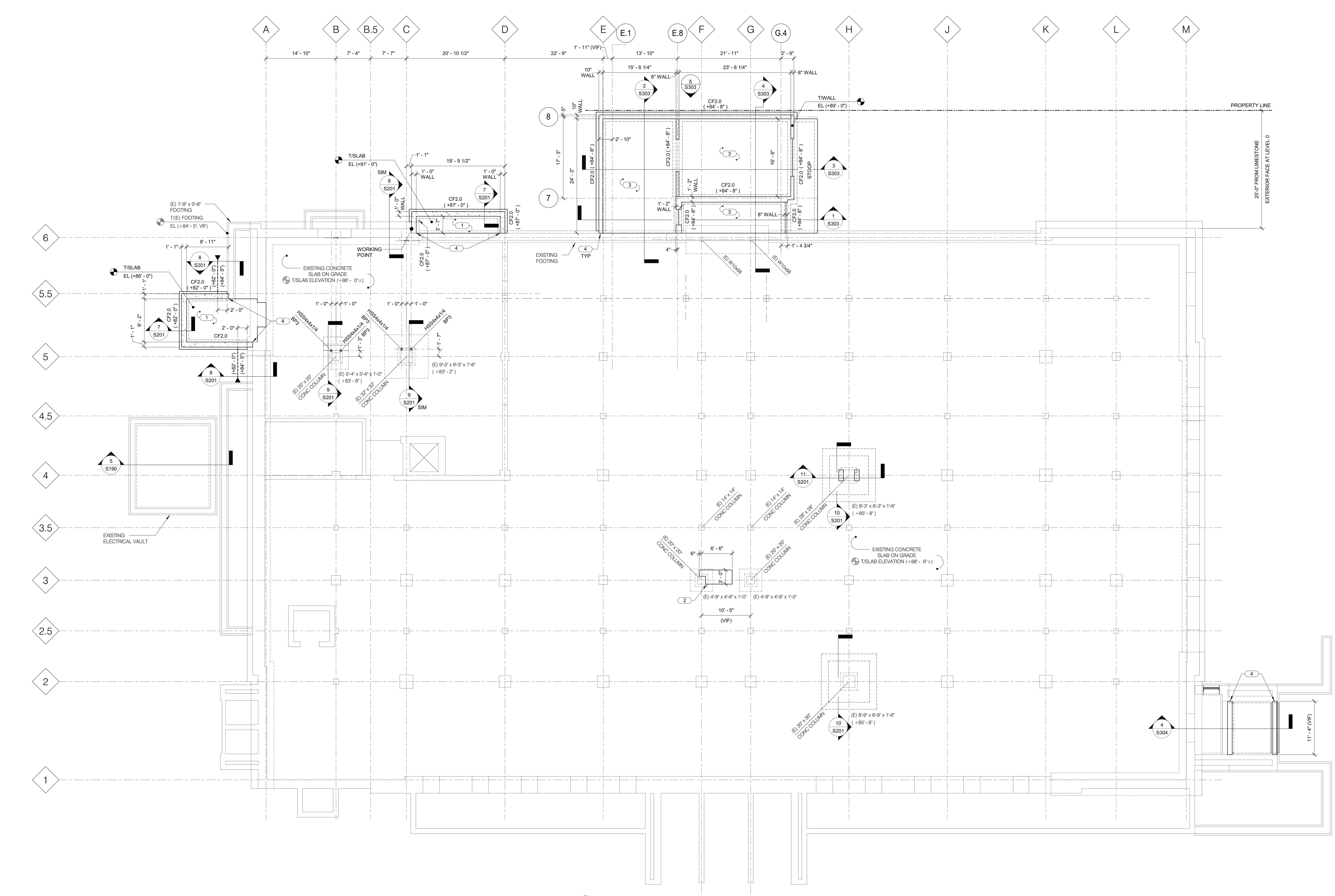

GROUND LEVEL FOUNDATION PLAN

1/8" = 1'-0"

NOTES:

1. TOP OF FOOTING ELEVATION (+86' - 0"), UNO. MATCH BOTTOM OF EXISTING FOOTING ELEVATION.
2. TOP OF FOUNDATION WALL ELEVATION (+88' - 8").
3. SF# AND CF# INDICATES SPREAD AND CONTINUOUS FOOTINGS. REFER TO THIS SHEET FOR SCHEDULES.
4. REFER TO 1 AND 2/S201 FOR TYPICAL SLAB ON GRADE CONSTRUCTION DETAILS.
5. REFER TO 6/S201 FOR TYPICAL SLAB ON GRADE PATCHING DETAIL. REFER TO ARCHITECTURAL DRAWINGS FOR PATCH LOCATIONS.
6. ▶---------- INDICATES FOOTING STEP. REFER TO 12/S201 FOR FOOTING STEP DETAIL.

KEYNOTES:

1. 4" CONCRETE SLAB ON GRADE WITH 6X6 - W2.1XW2.1 WWR.
2. DEMO EXISTING CONCRETE AND REPLACE WITH 12" THICKENED WITH (3) #5 BARS IN LONG DIRECTION SLAB FOR STAIR STRINGER SUPPORT. DOWEL TO EXISTING SLAB PER 6/S201.
3. 5" CONCRETE SLAB ON GRADE WITH 6X6 - W2.1XW2.1 WWR. T/SLAB ELEVATION SLOPES, REFER TO ARCHITECTURAL DRAWINGS.
4. DOWEL HORIZONTAL WALL AND FOOTING REINFORCING TO EXISTING WITH ADHESIVE PER S000 AND 4" EMBEDMENT. PROVIDE WATERSTOP AT VERTICAL JOINT. BOTTOM OF NEW FOOTING ELEVATION TO MATCH BOTTOM OF EXISTING FOOTING ELEVATION.
5. SLEEVE UTILITY THROUGH FOUNDATION PER 13/S201. COORDINATE SIZE AND LOCATION WITH TRADE CONTRACTOR.

CONTINUOUS FOOTING SCHEDULE

MARK	WIDTH	THICKNESS	REINFORCING LONG DIRECTION	REINFORCING SHORT DIRECTION
CF2.0	2' - 0"	1' - 0"	(3) #5	WALL DOWELS

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ARCHITECT SEAL

Signature: ______
Print Names: ______
Date: ______ License No: ______

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FOUNDATION PLAN

EXHIBIT H

S100

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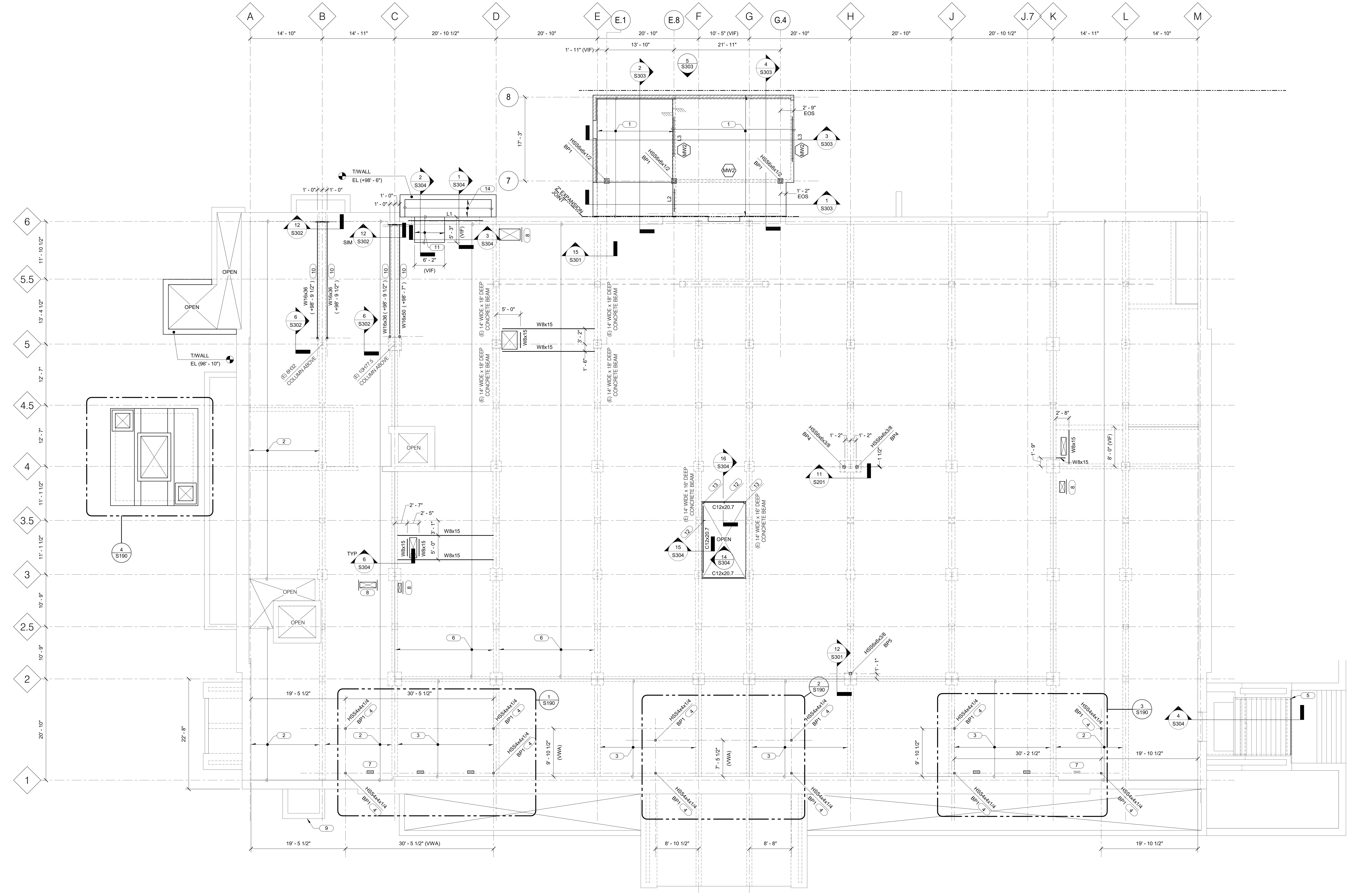

LINTEL SCHEDULE			
MARK	MEMBER BEARING, EACH END	MEMBER SIZE	REFERENCE DETAIL
L1	8"	W14x43 + 3/8" BOTTOM PLATE	1/S305
L2	8"	8" DEEP BOND BEAM WITH (2) #5 BARS	2/S301
L3	8"	16" DEEP BOND BEAM WITH (2) #5 BARS	2/S301
L4	8"	W8x24 + 3/8" BOTTOM PLATE	1/S301

NOTES:

1. REFER TO 8/S301 FOR TYPICAL MASONRY OPENING DETAIL.

MASONRY WALL REINFORCING SCHEDULE			
MARK	WALL THICKNESS	VERTICAL WALL REINFORCING SIZE AND SPACING	HORIZONTAL WALL REINFORCING SIZE AND SPACING
MW1	8"	(2) #5 @ 8" OC	TYPICAL
MW2	8"	#5 @ 24" OC	TYPICAL

NOTES:

1. TYPICAL HORIZONTAL REINFORCING IS AS PER SPECIFICATIONS. IT IS INTENDED TO BE A 'DUROWAL - TRUSS TYPE' OR EQUIVALENT.
2. 'GROUT ALL CORES' INDICATES EVERY REINFORCED CORE AND UNREINFORCED CORE.
3. REINFORCED CORES ARE ALWAYS GROUTED.

PLAN NORTH

1 LEVEL ONE FLOOR FRAMING PLAN

1/8" = 1'-0"

NOTES:

1. L# INDICATES LINTEL IN STRUCTURAL MASONRY WALL. REFER TO THIS SHEET FOR SCHEDULE. REFER TO GENERAL NOTES FOR LINTELS IN NON-STRUCTURAL WALLS.
2. MW# INDICATES MASONRY WALL - REFER TO THIS SHEET FOR SCHEDULE.

KEYNOTES:

1. 8" CONCRETE SLAB WITH #5 @ 8" OC, EACH WAY, TOP AND BOTTOM. REFER TO SECTIONS FOR TOP OF SLAB ELEVATIONS.
   REINFORCEMENT PLACEMENT SEQUENCE:
   EAST-WEST BOTTOM LAYER BARS
   NORTH-SOUTH BOTTOM LAYER BARS
   NORTH-SOUTH TOP LAYER BARS
   EAST-WEST TOP LAYER BARS
2. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 8" DEEP JOIST RIBS = 10" TOTAL THICKNESS.
3. EXISTING 7" THICK CONCRETE SLAB.
4. COLUMN TO EXTEND DOWN TO STRUCTURAL SLAB BELOW, T/SLAB = (+99' - 8") VIF. PATCH FLOORING PER ARCHITECTURAL DRAWINGS.
5. NEW CONCRETE STAIR TO SUPPORT STONE STAIR TREADS. COORDINATE WITH BE-SERIES SHEETS AND EXISTING CONDITIONS.
6. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2 1/2" THICK SLAB + 10" DEEP JOIST RIBS = 12 1/2" TOTAL THICKNESS.
7. LOCATE NEW MECHANICAL OPENING BETWEEN EXISTING CONCRETE JOIST RIBS.
8. PROVIDE ANGLE REINFORCING AT NEW MECHANICAL OPENING. ANCHOR TO ADJACENT CONCRETE JOIST RIBS PER 9/S304. LOCATE OPENING SO ONLY ONE EXISTING JOIST IS CUT.
9. DEMO DAMAGED MASONRY PORTION OF EXISTING AREA WAY AND REPLACE WITH NEW CONCRETE CURB, DOWELED INTO EXISTING CONCRETE BELOW.
10. REFER TO DETAIL 13/S302 FOR TYPICAL BEAM BRACING DETAIL.
11. INFILL EXISTING DOCK LEVELER PIT WITH 2 1/2" NORMAL WEIGHT CONCRETE ON 1 1/2" (18 GA) CONFORM DECK, SINGLE SPAN, WITH 6x6 -W1.4xW1.4 WWR. TOTAL THICKNESS = 4". TOP OF SLAB ELEVATION TO MATCH EXISTING.
12. SINGLE PLATE HANGER DOWN TO STAIR LANDING, REFER TO ARCHITECTURAL DRAWINGS.
13. DOUBLE PLATE HANGER DOWN TO STAIR LANDING, REFER TO ARCHITECTURAL DRAWINGS. WELD PLATES TOGETHER TO FORM L-SHAPE.
14. 1 1/2" BAR GRATING. SELECT BAR THICKNESS AND SPACING TO SUPPORT 100 PSF LIVE LOAD.

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Print Names: ______
Date: ______ License No: ______

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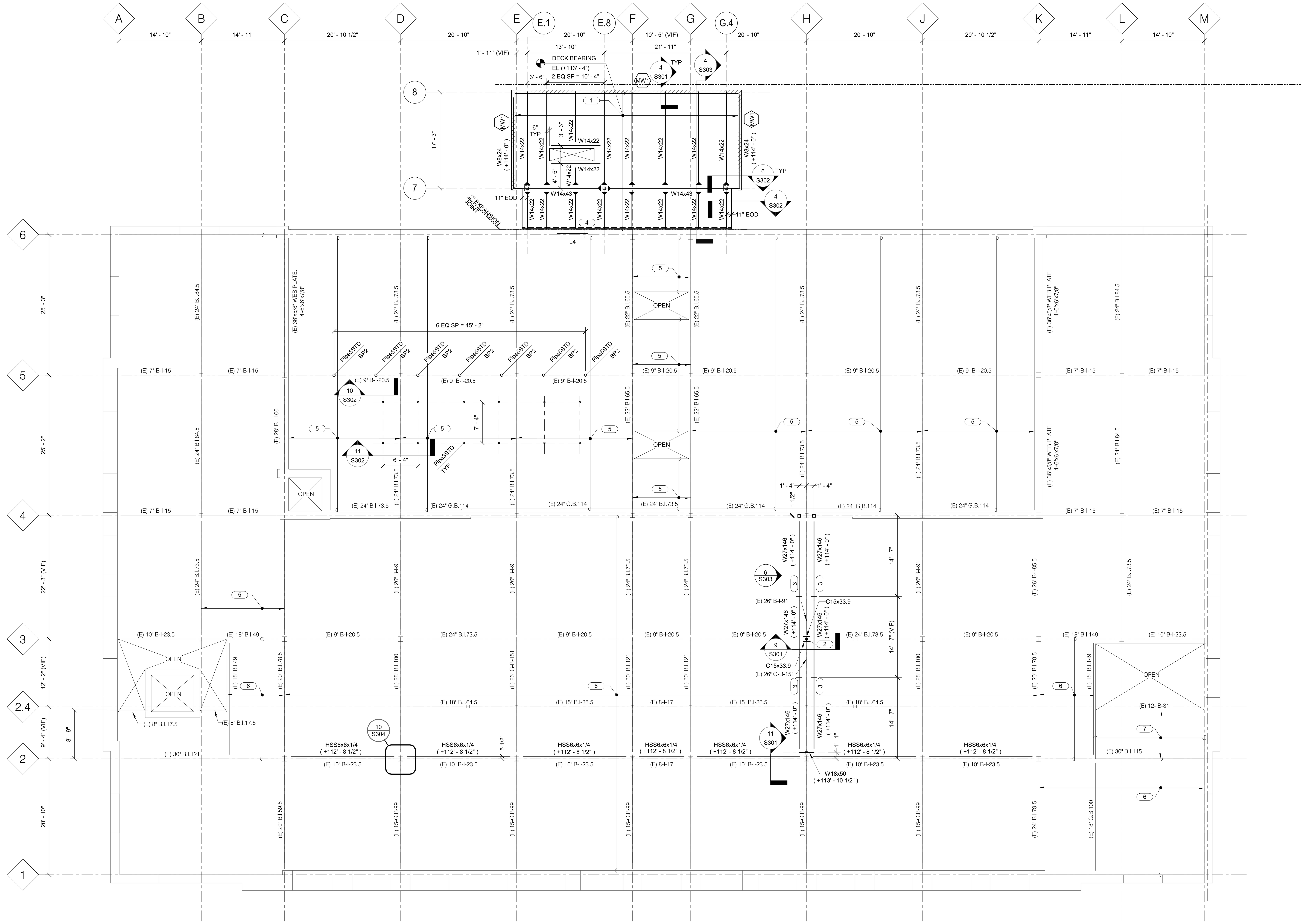
FIRST FLOOR FRAMING PLAN

EXHIBIT H

S101



1 LEVEL TWO FLOOR FRAMING PLAN

PLAN NORTH

1/8" = 1'-0"

NOTES:

1. L# INDICATES LINTEL IN STRUCTURAL MASONRY WALL. REFER TO S101 FOR SCHEDULE. REFER TO GENERAL NOTES FOR LINTELS IN NON-STRUCTURAL WALLS.
2. (MW#) INDICATES MASONRY WALL - REFER TO S101 FOR SCHEDULE.
3. BP# INDICATES BASE PLATE. REFER S302 FOR BASE PLATE DETAILS.

KEYNOTES:

1. 1 1/2" (20 GA) TYPE B STEEL ROOF DECK, 2-SPAN MINIMUM. FASTEN WITH 36/4 (1) PATTERN PER 2/S302.
2. CONTRACTOR TO DESIGN, FURNISH AND INSTALL SHORING PRIOR TO DEMOLITION (DL = 100K).
3. REFER TO 10/S301 FOR BEAM SPLICE DETAIL.
4. L4x4x1/4 x CONT DECK EDGE ANGLE.
5. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 8" DEEP JOIST RIBS = 10" TOTAL THICKNESS. TOP OF EXISTING STRUCTURAL SLAB (+116'-4 3/4"), VIF.
6. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 10" DEEP JOIST RIBS = 12" TOTAL THICKNESS.

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Print Names:
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SECOND FLOOR FRAMING PLAN

EXHIBIT H

S102

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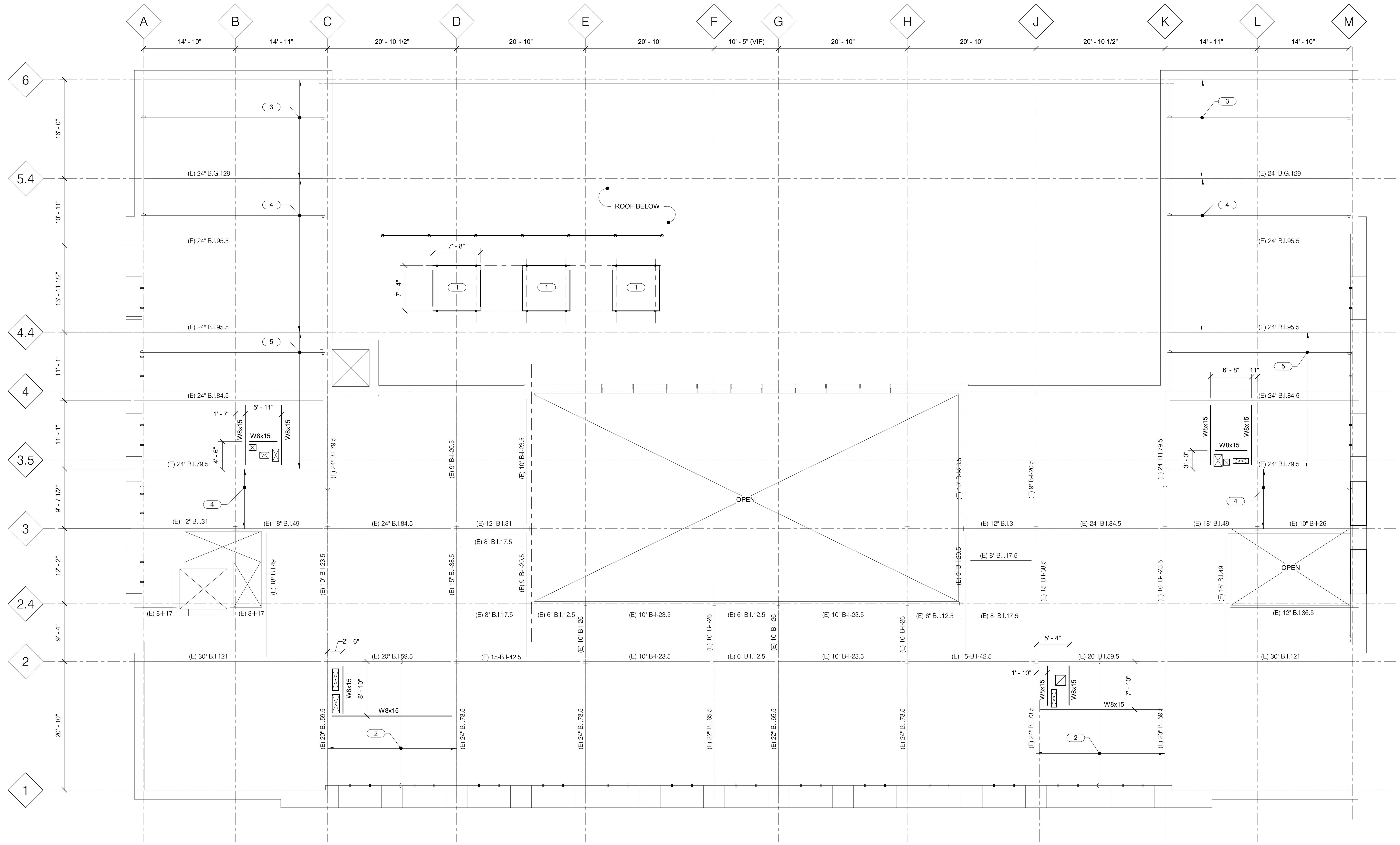

LEVEL THREE FLOOR FRAMING PLAN

PLAN NORTH

1

1/8" = 1'-0"

KEYNOTES:

1. CONDENSING UNIT: PROVIDE PLATFORM WITH W8x15 SUPPORT BEAMS ON ALL SIDES. LOCATE PLATFORM TO PROVIDE 18" CLEAR UNDERNEATH. COORDINATE PLATFORM DIMENSIONS WITH MECHANICAL CONTRACTOR AND SELECTED EQUIPMENT.
2. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 10" DEEP JOIST RIBS = 12" TOTAL THICKNESS.
3. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 8" DEEP JOIST RIBS = 10" TOTAL THICKNESS.
4. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 6" DEEP JOIST RIBS = 8" TOTAL THICKNESS.
5. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 4" DEEP JOIST RIBS = 6" TOTAL THICKNESS.

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Print Names:
Date: License No:

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PROJECT PHASE BID SET

DRAWN BY: PRIPAN CHECKED BY: ABBPER

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REFERENCE SCALE IN INCHES
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THIRD FLOOR FRAMING PLAN

EXHIBIT H

S103

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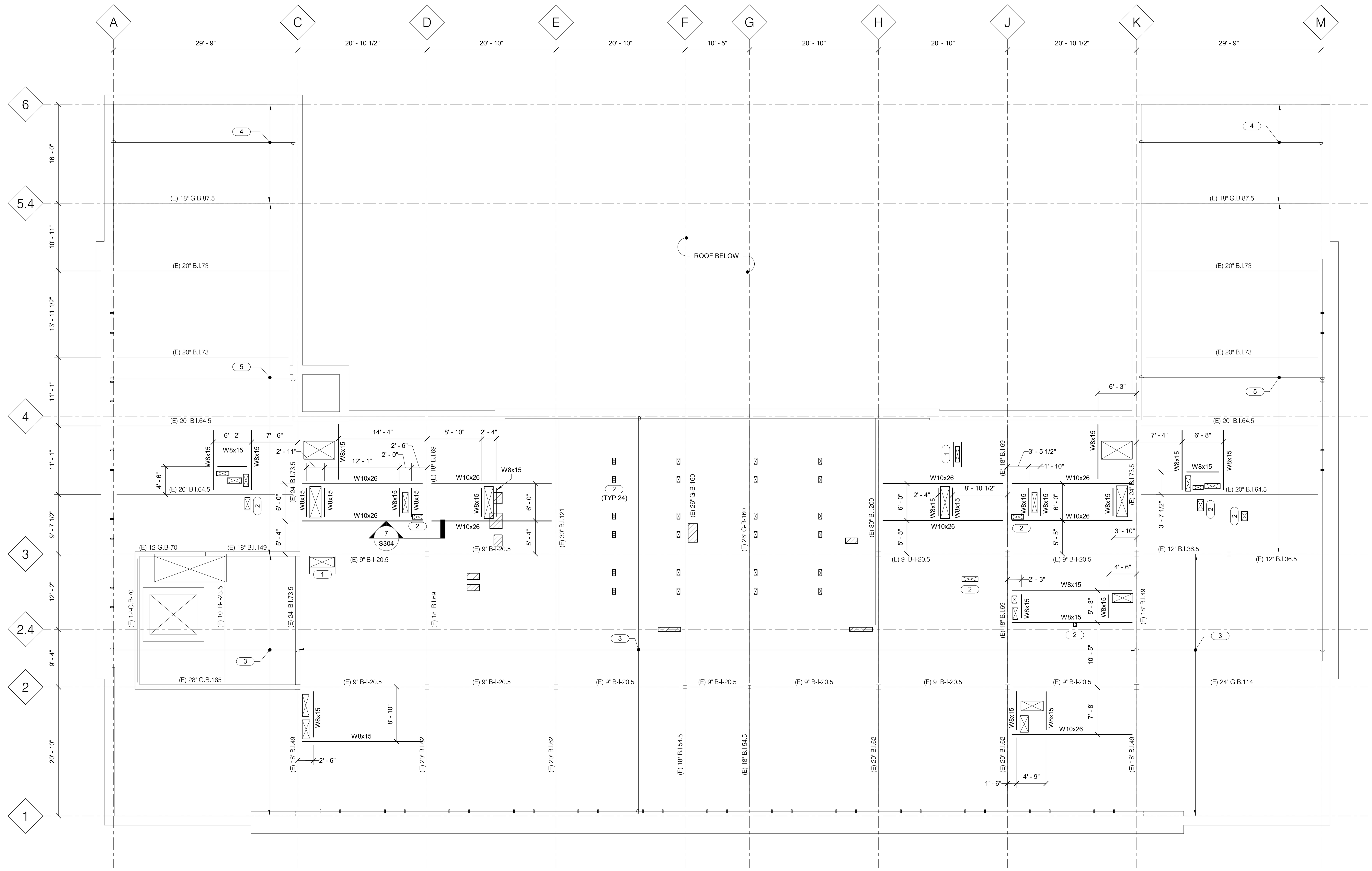

ROOF SLAB FRAMING PLAN

1 — 1/8" = 1'-0"

PLAN NORTH

KEYNOTES:

1. PROVIDE ANGLE REINFORCING AT NEW MECHANICAL OPENING. ANCHOR TO ADJACENT CONCRETE JOIST RIBS. PER #/S###. LOCATE OPENING SO ONLY ONE EXISTING JOIST IS CUT.
2. LOCATE NEW MECHANICAL OPENING BETWEEN EXISTING CONCRETE JOIST RIBS.
3. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 8" DEEP JOIST RIBS = 10" TOTAL THICKNESS.
4. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 6" DEEP JOIST RIBS = 8" TOTAL THICKNESS.
5. EXISTING ONE-WAY CONCRETE JOIST AND CLAY TILE FLOOR SLAB: 5" WIDE JOISTS AT 17" OC. 2" THICK SLAB + 4" DEEP JOIST RIBS = 6" TOTAL THICKNESS.

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ROOF SLAB FRAMING PLAN

EXHIBIT H

S104

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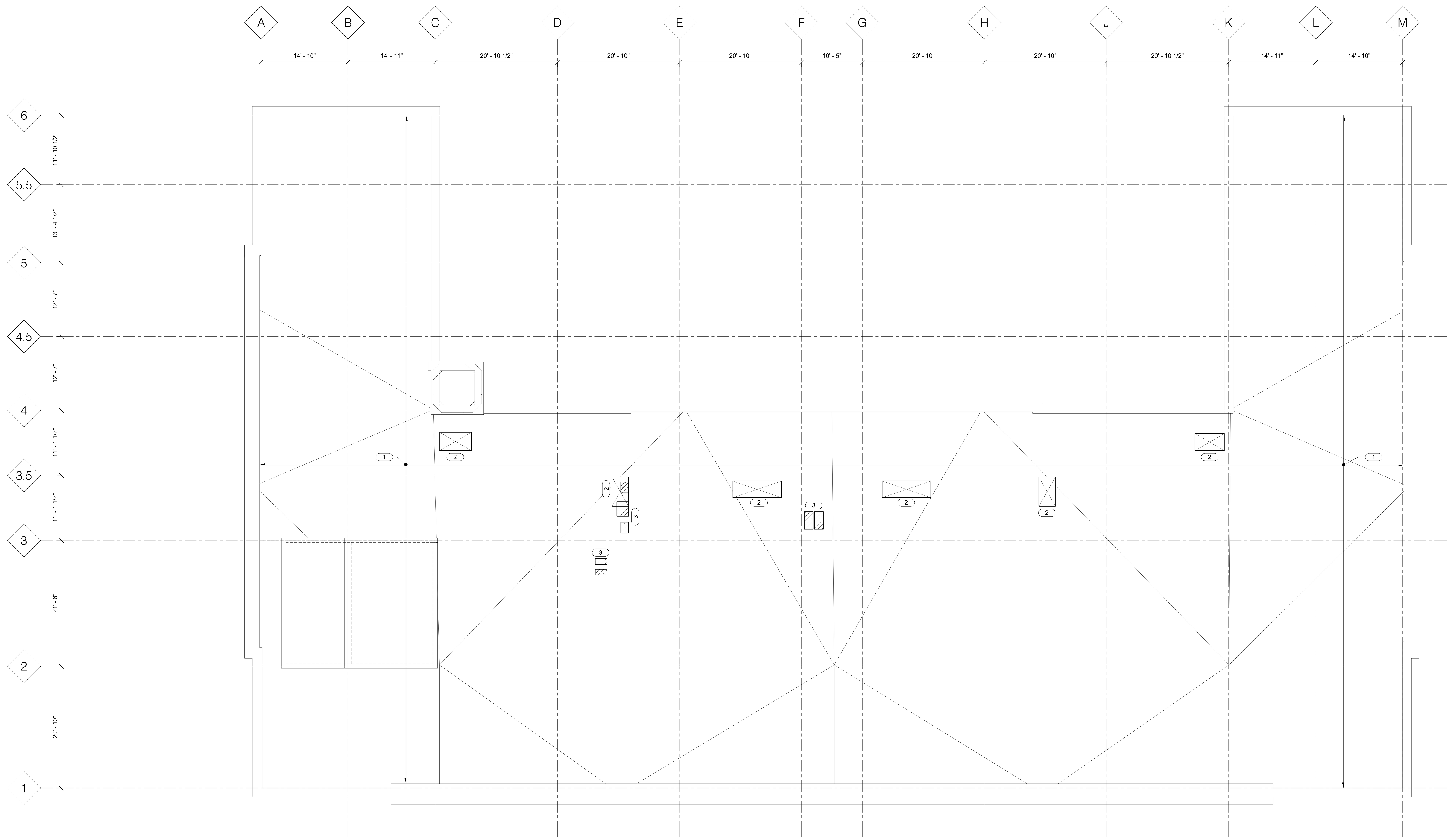

PLAN NORTH

1

ROOF FRAMING PLAN

1/8" = 1'-0"

KEYNOTES:

1. EXISTING WOOD ROOF FRAMING: 2" WOOD PLANK DECKING SUPPORTED BY 4x4 WOOD BEAMS AND POSTS SPACED 5'-0" OC.
2. NEW ROOF OPENING: SUPPORT EXISTING WOOD ROOF FRAMING PER 11/S304. COORDINATE OPENING LOCATION WITH TRADE CONTRACTOR AND EXISTING FRAMING LOCATIONS.
3. EXISTING ROOF OPENING: PATCH OPENING WITH SALVAGED MATERIAL FROM LOW ROOF DEMO. REFER TO 13/S304.

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Print Names: ______

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ROOF FRAMING PLAN

EXHIBIT H

S105

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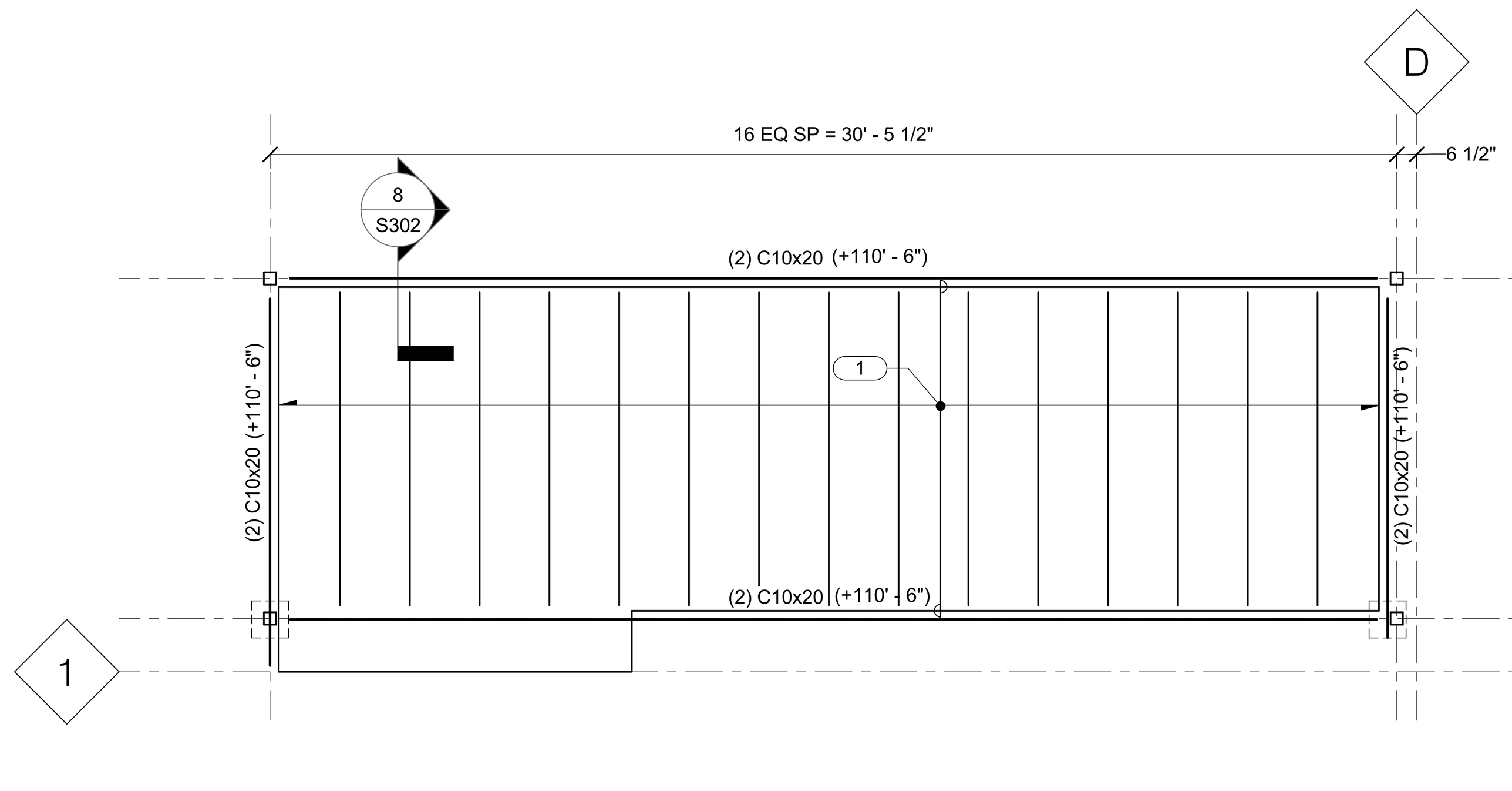
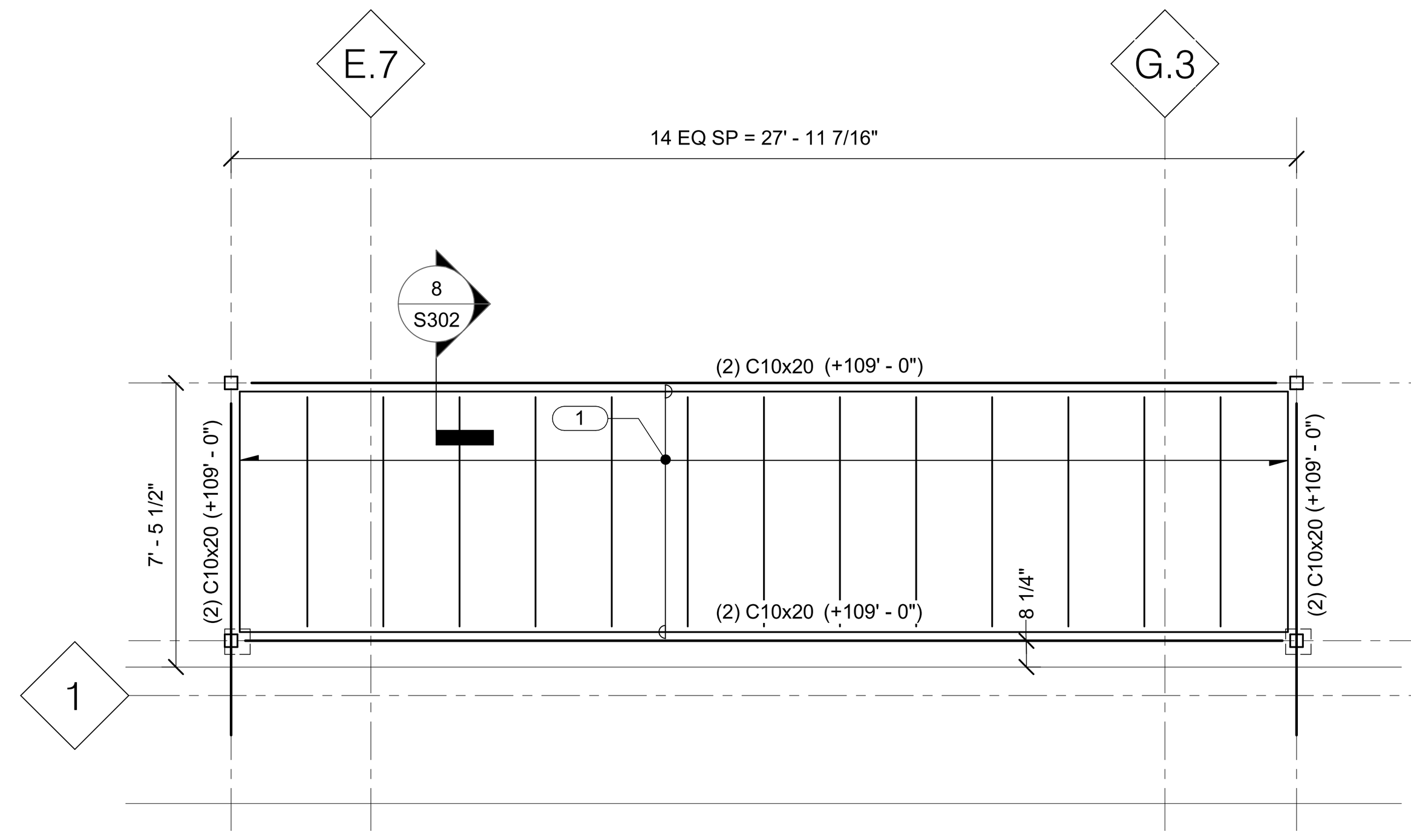


ENLARGED CEILING FRAMING PLAN
PLAN NORTH 1

1/4" = 1'-0"

KEYNOTES:

1 1/2" WOOD SHEATHING SUPPORTED BY 6" DEEP COLD FORM JOISTS.

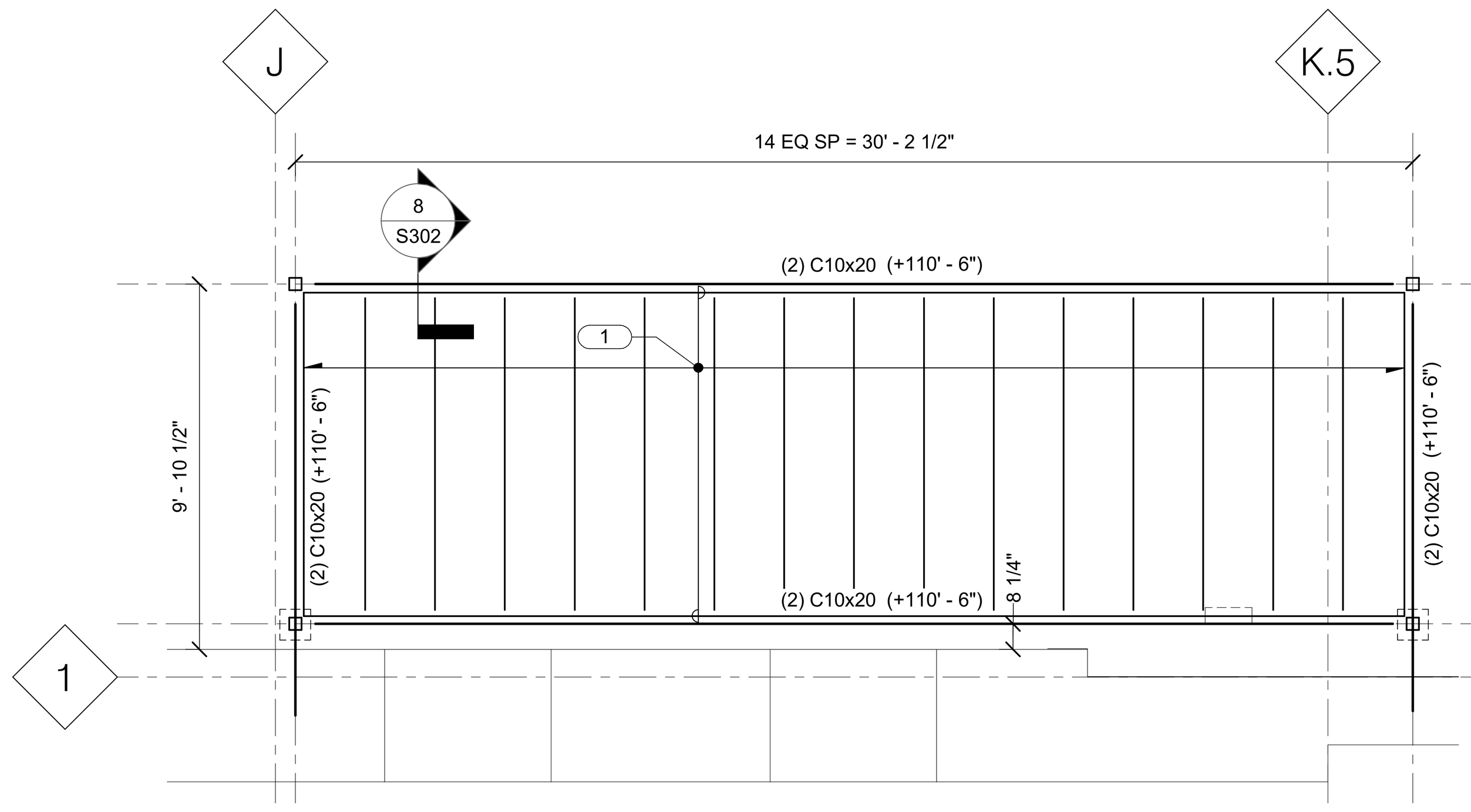


ENLARGED CEILING FRAMING PLAN
PLAN NORTH 2

1/4" = 1'-0"

KEYNOTES:

1 1/2" WOOD SHEATHING SUPPORTED BY 6" DEEP COLD FORM JOISTS.

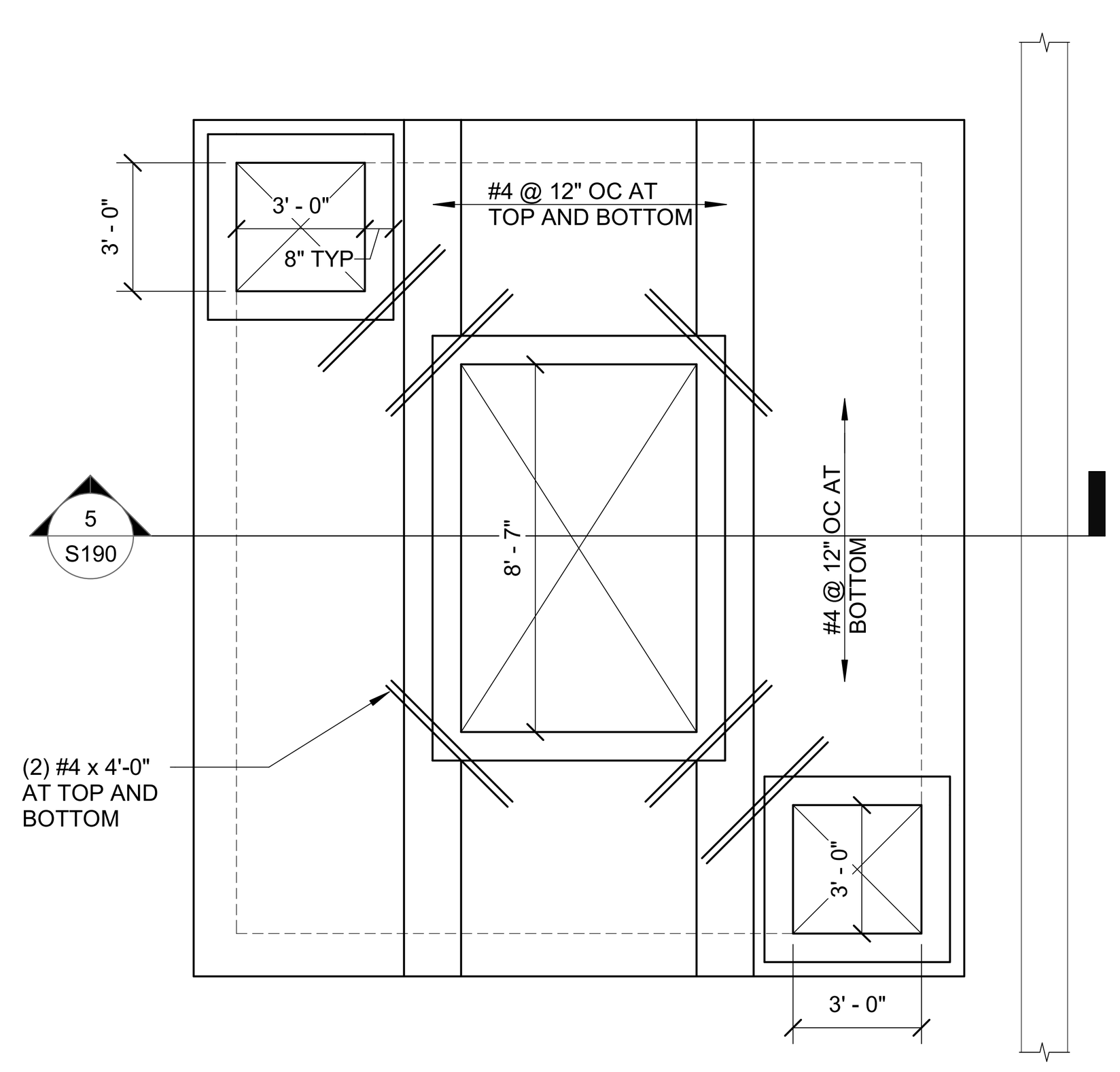


ENLARGED CEILING FRAMING PLAN
PLAN NORTH 3

1/4" = 1'-0"

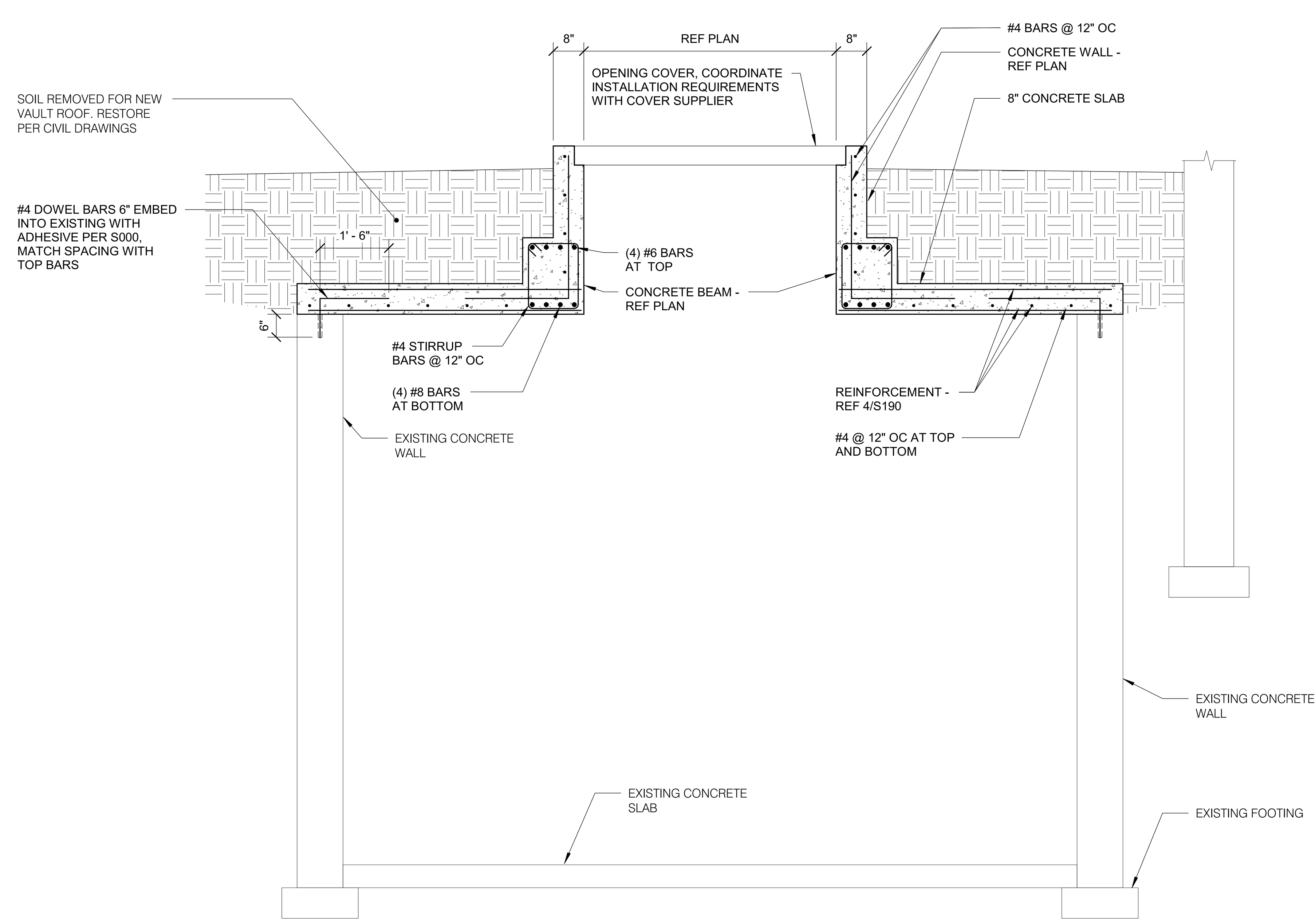
KEYNOTES:

1 1/2" WOOD SHEATHING SUPPORTED BY 6" DEEP COLD FORM JOISTS.



ELECTRICAL VAULT ROOF FRAMING PLAN
PLAN NORTH 4

1/4" = 1'-0"



SECTION THROUGH ELECTRICAL VAULT ROOF
5

1/2" = 1'-0"

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REFERENCE SCALE IN INCHES 0 1 2 3

ENLARGED PLANS

EXHIBIT H

S190

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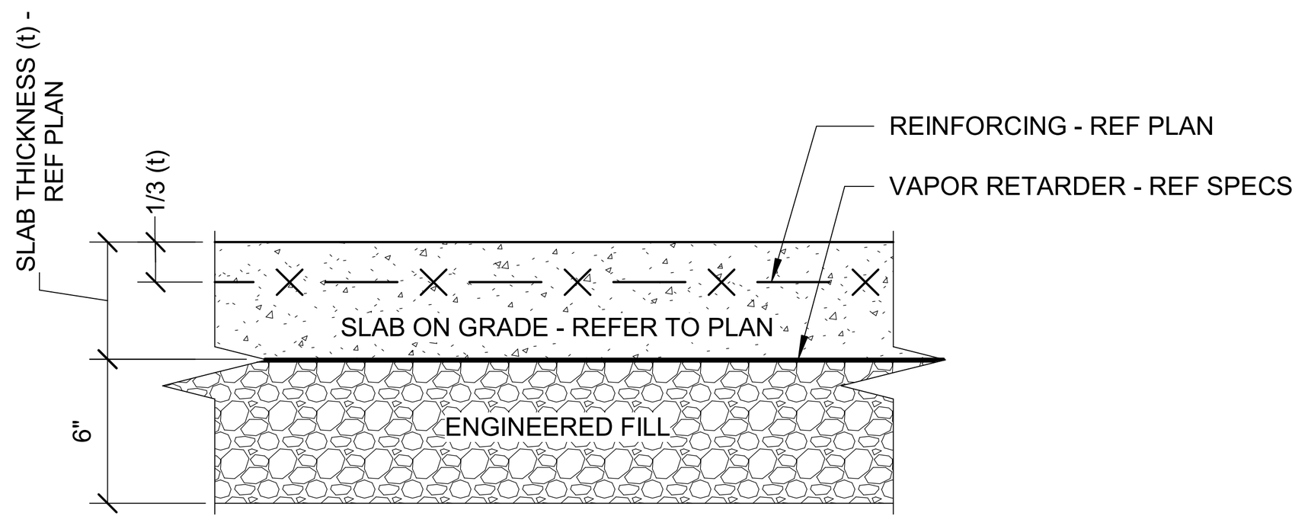
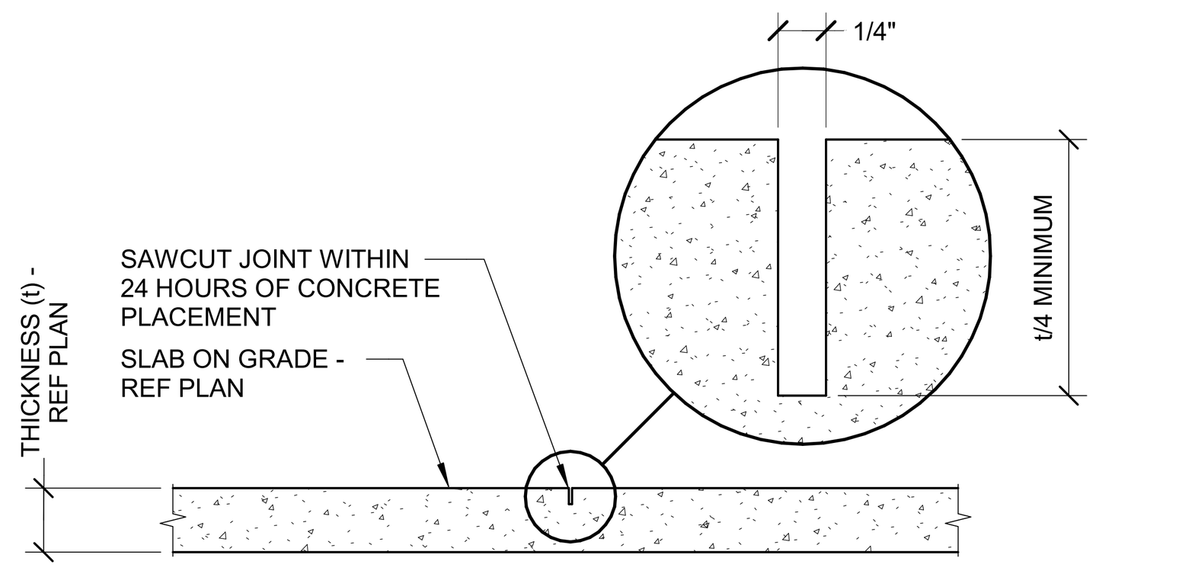


1 TYPICAL SLAB ON GRADE SECTION

1 1/2" = 1'-0"

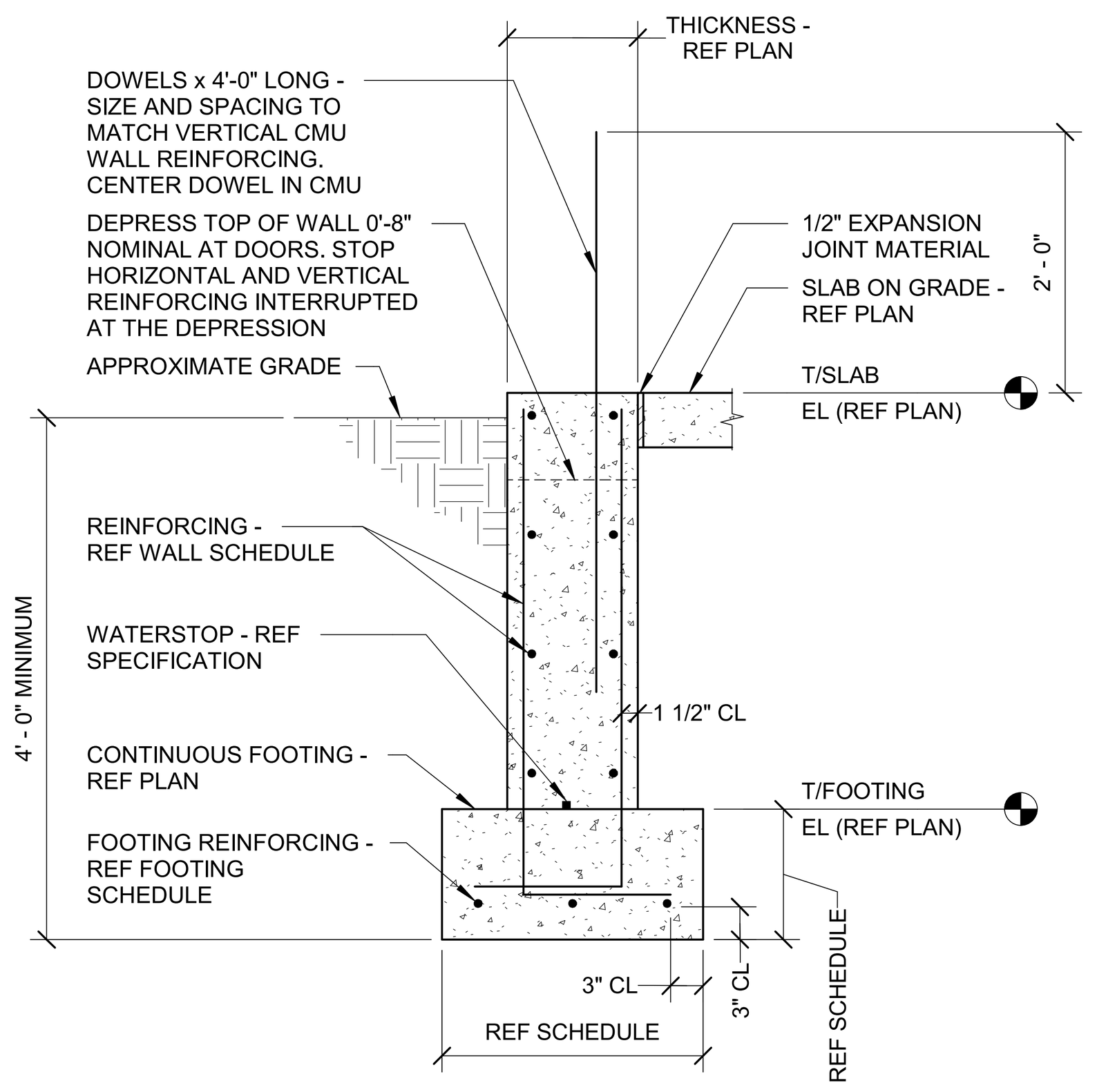
NOTES:

1. REFERENCE SPECIFICATIONS FOR MATERIAL AND COMPACTION REQUIREMENTS.



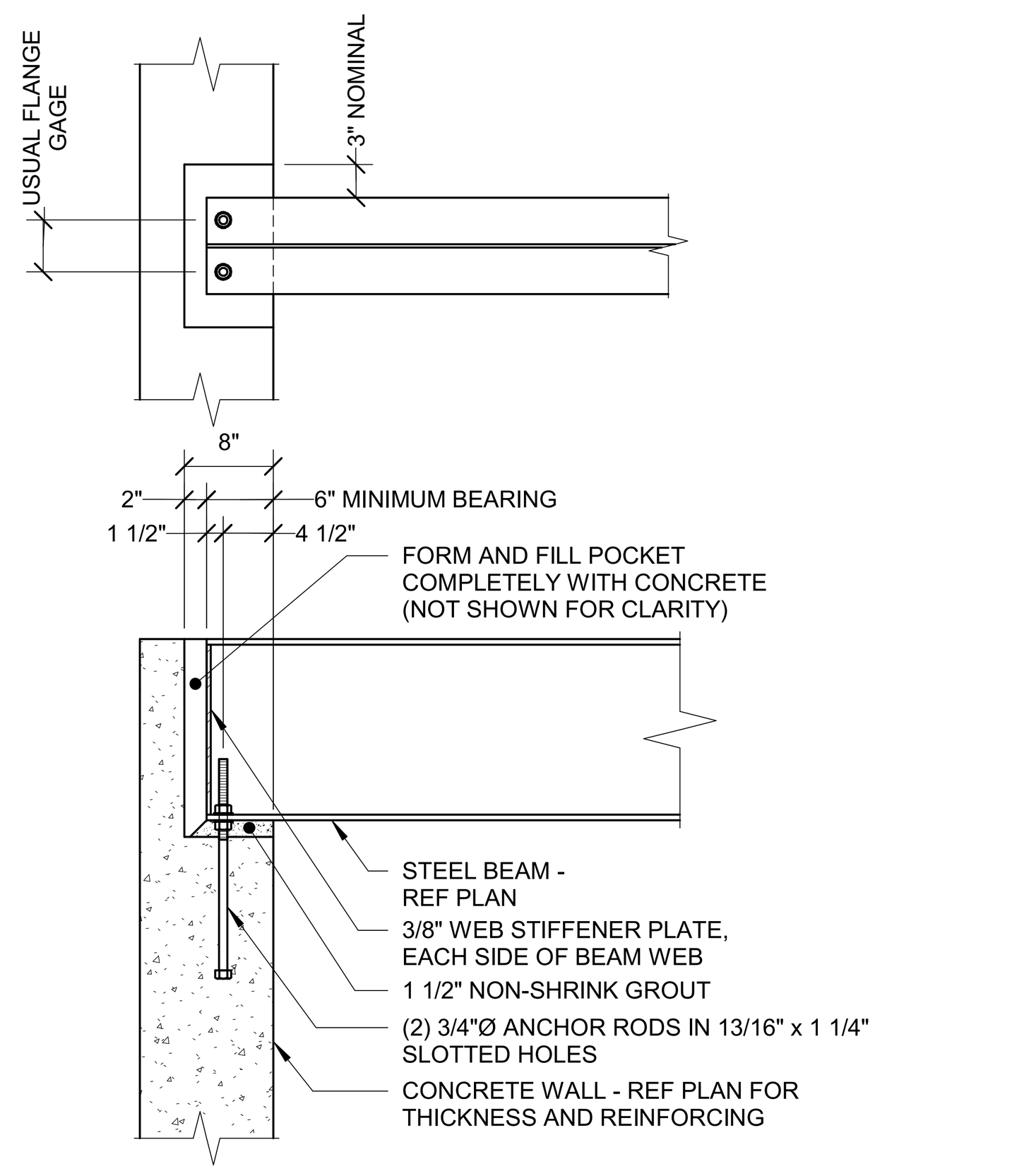
2 TYPICAL SLAB ON GRADE CONTROL JOINT

1" = 1'-0"



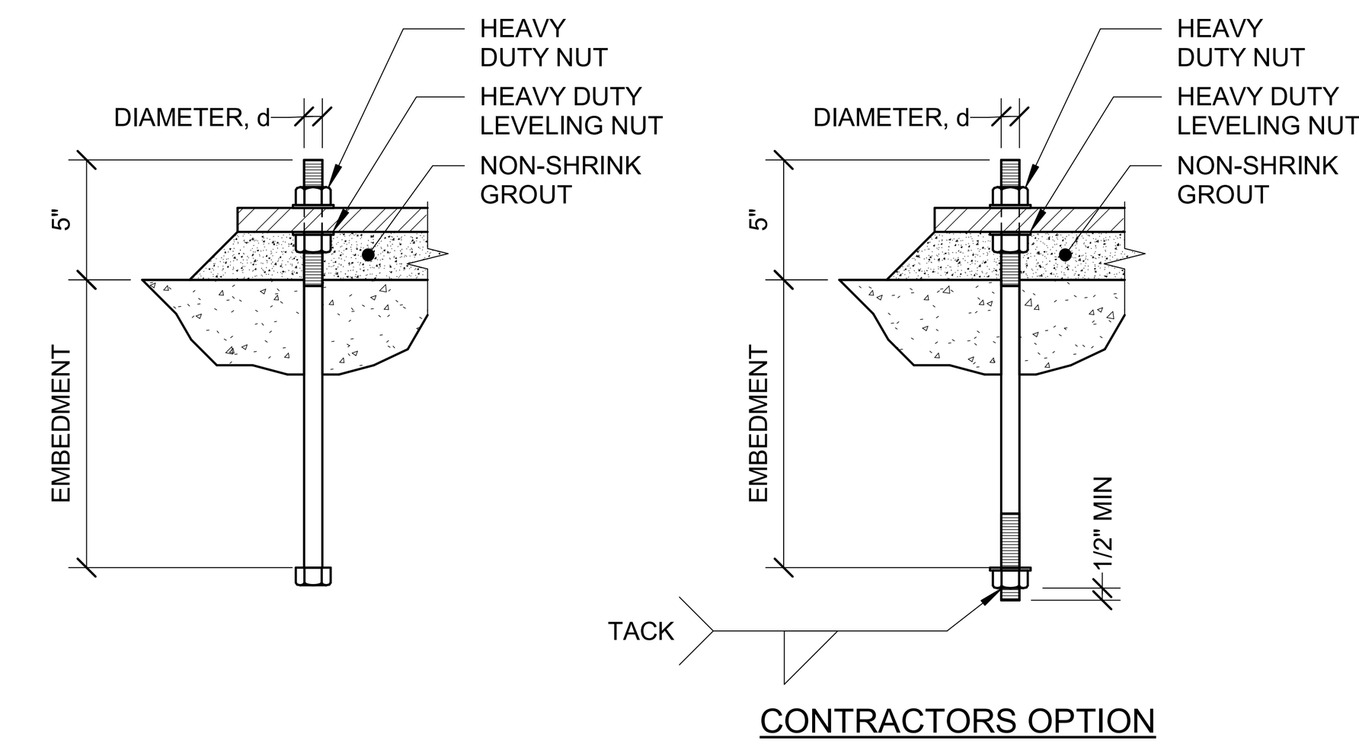
3 TYPICAL FOUNDATION WALL

3/4" = 1'-0"



4 TYPICAL BEAM POCKET DETAIL

3/4" = 1'-0"

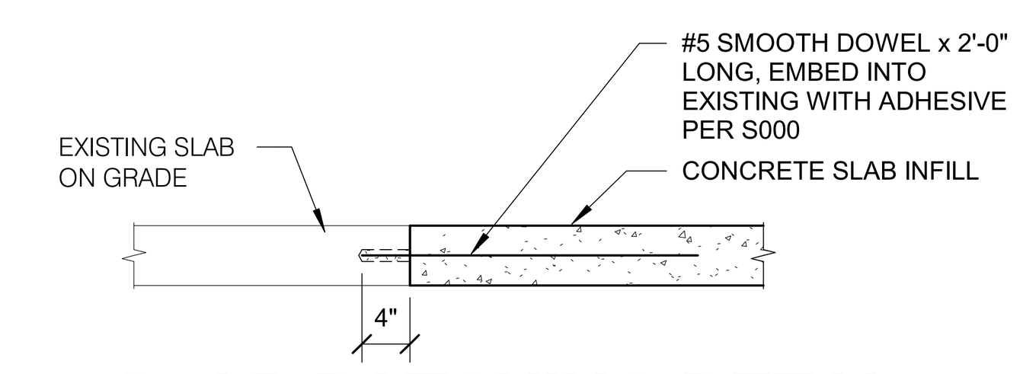


5 TYPICAL ANCHOR ROD

1 1/2" = 1'-0"

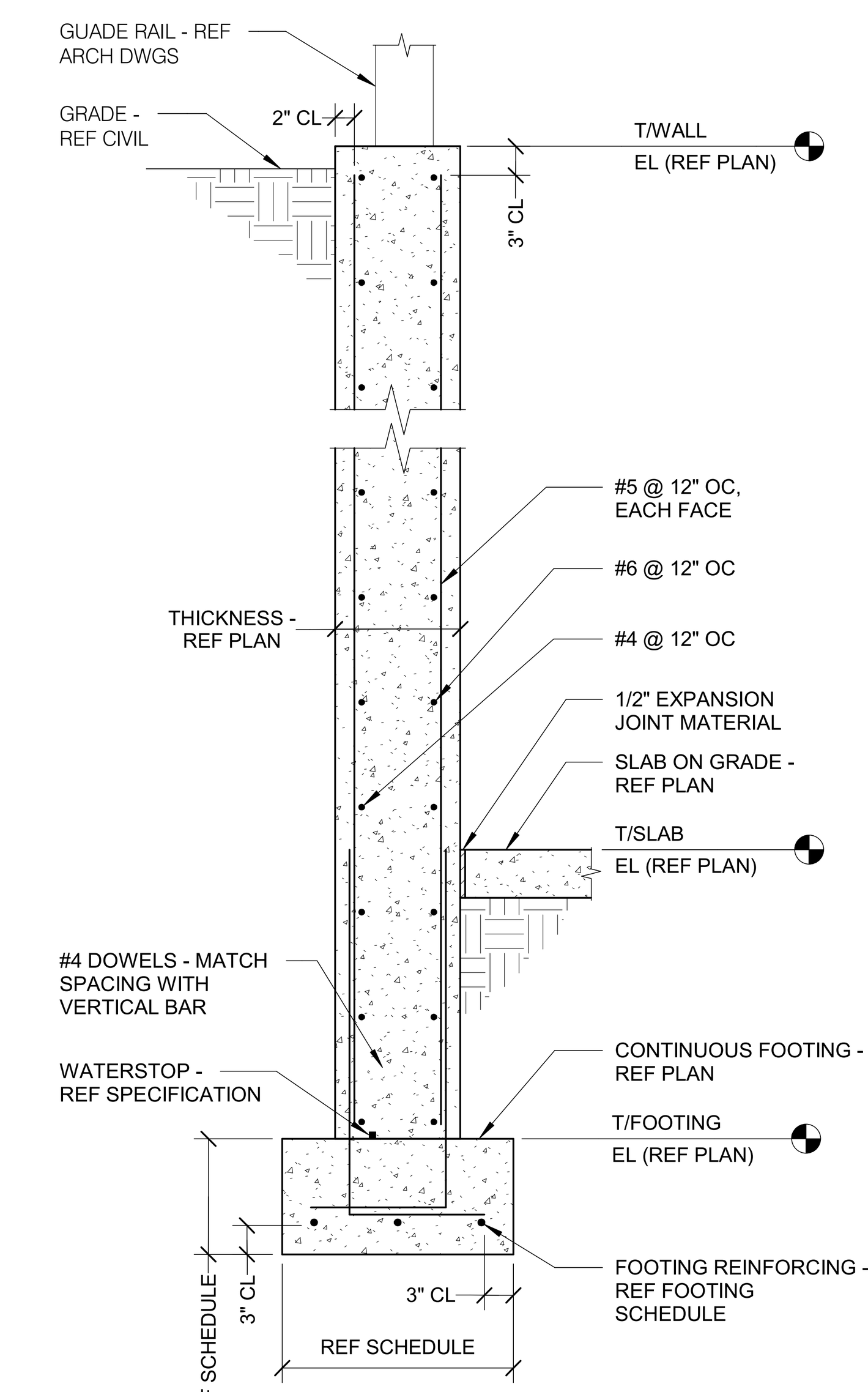
NOTES:

1. REFERENCE BASE PLATE DETAILS FOR DIAMETER AND EMBEDMENT.
2. REFERENCE GENERAL NOTES FOR MATERIAL REQUIREMENTS.
3. ANCHOR RODS SHALL BE SET PRIOR TO PLACEMENT OF CONCRETE.
4. PROTECT ANCHOR RODS FROM DAMAGE.
5. ANCHOR SHALL BE SET SO AS NOT TO VARY FROM THE DIMENSIONS SHOWN ON THE ERECTION DRAWINGS BY MORE THAN THE FOLLOWING:
   A. 1/8" CENTER TO CENTER OF ANY TWO RODS WITHIN AN ANCHOR ROD GROUP.
   B. 1/4" CENTER TO CENTER OF ADJACENT ANCHOR ROD GROUPS.
   C. ELEVATION OF THE TOP OF ANCHOR RODS ± 1/2".
   D. MAXIMUM ACCUMULATION OF 1/4" PER HUNDRED FEET ALONG THE ESTABLISHED COLUMN LINE.
   E. 1/4" FROM THE CENTER OF ANY ANCHOR ROD GROUP TO THE ESTABLISHED COLUMN LINE THROUGH THAT GROUP.
   F. REFERENCE AISC CODE OF STANDARD PRACTICE FOR ADDITIONAL INFORMATION.
6. SET ANCHOR RODS PERPENDICULAR TO BEARING SURFACE, UNLESS NOTED OTHERWISE.
7. PROVIDE 1 1/2" NON-SHRINK GROUT AT ALL BASE PLATES.



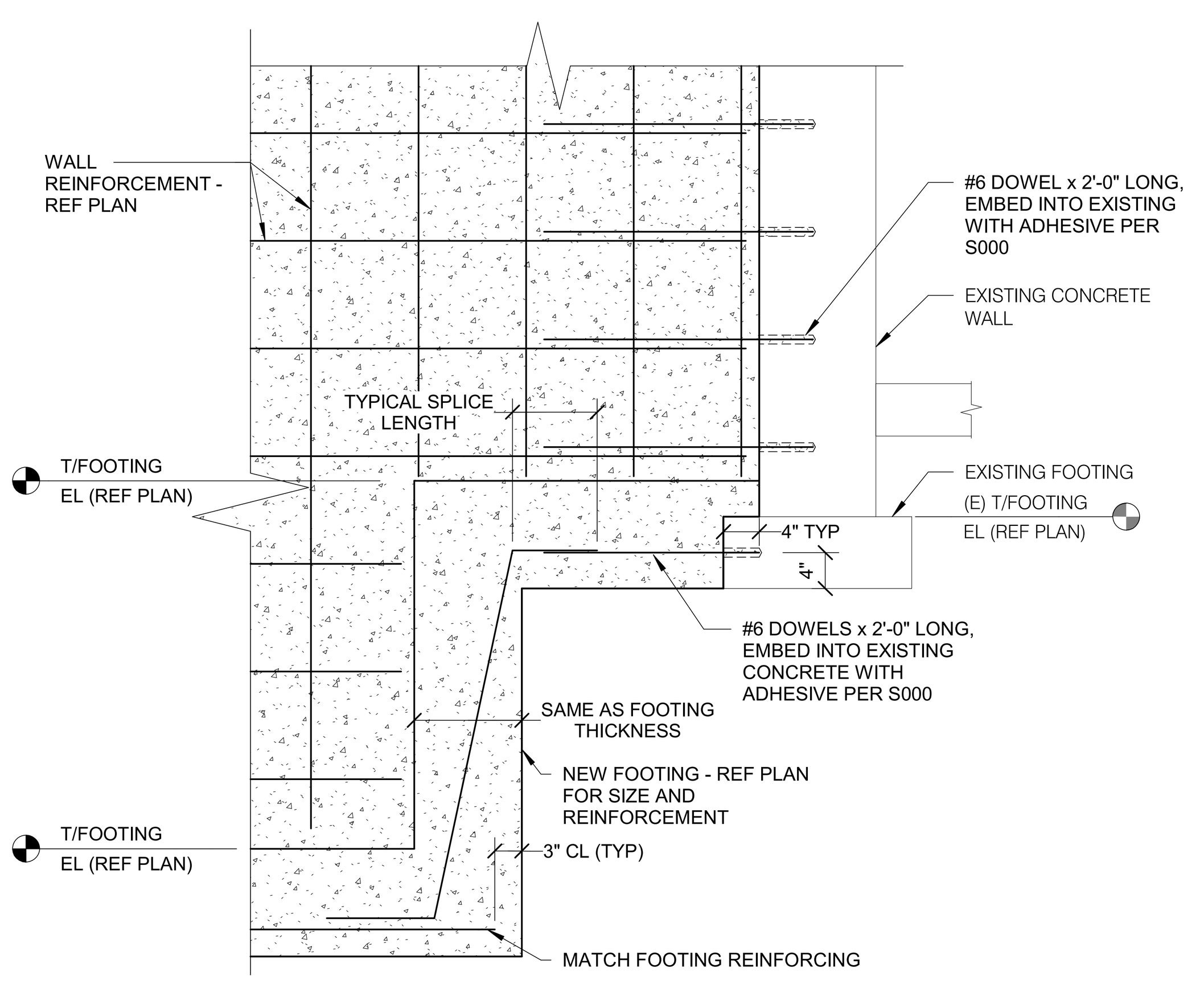
6 SLAB PATCHING DETAIL

3/4" = 1'-0"



7 AREA WELL WALL DETAIL

3/4" = 1'-0"

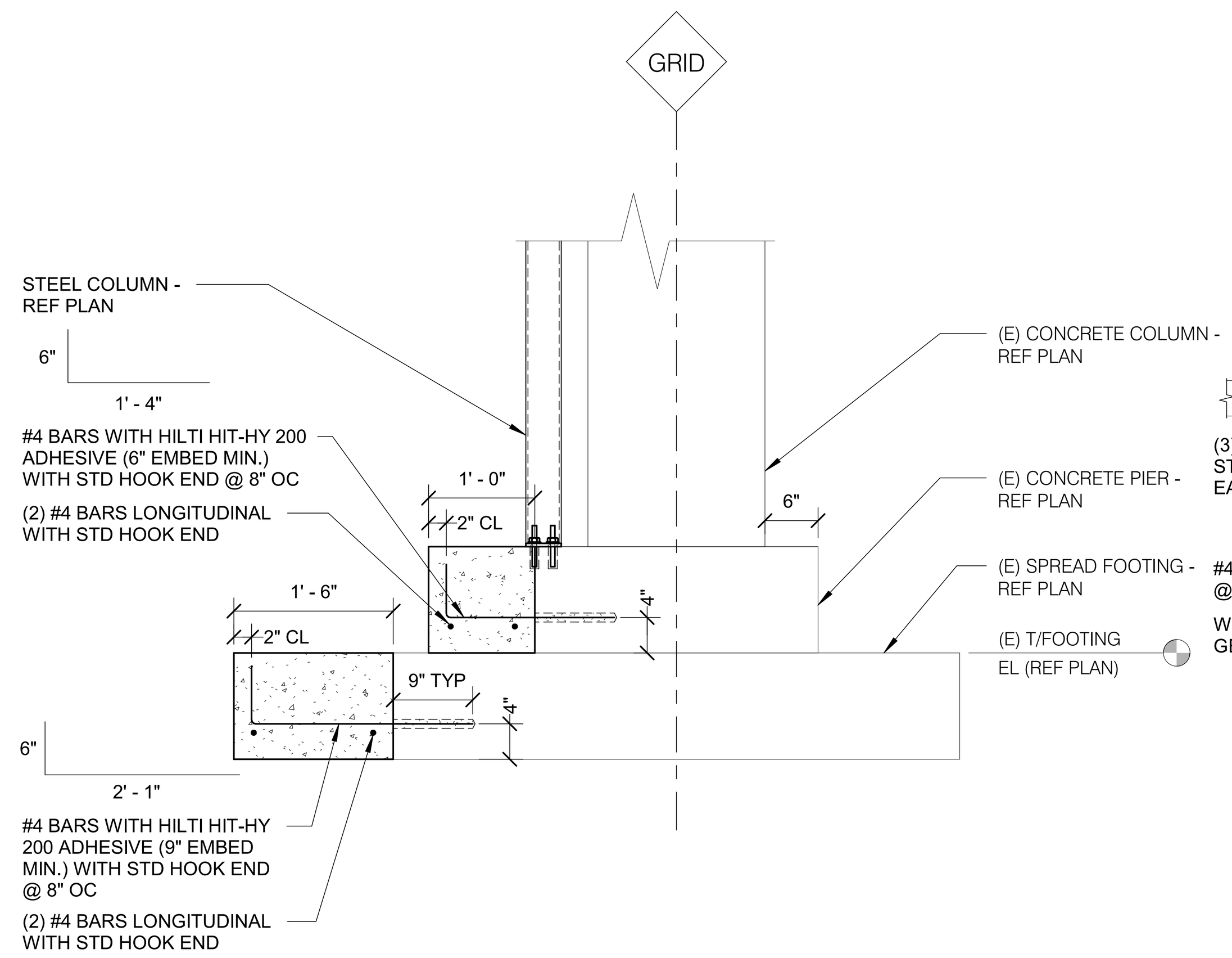


8 FOUNDATION DETAIL AT EXISTING

3/4" = 1'-0"

NOTES:

1. FOOTING REINFORCEMENT AND DOWEL NOT SHOWN FOR CLARITY.
2. AT SIM: NO STEP FOOTING. BOTTOM OF NEW FOOTING ELEVATION TO MATCH BOTTOM OF EXISTING FOOTING ELEVATION.

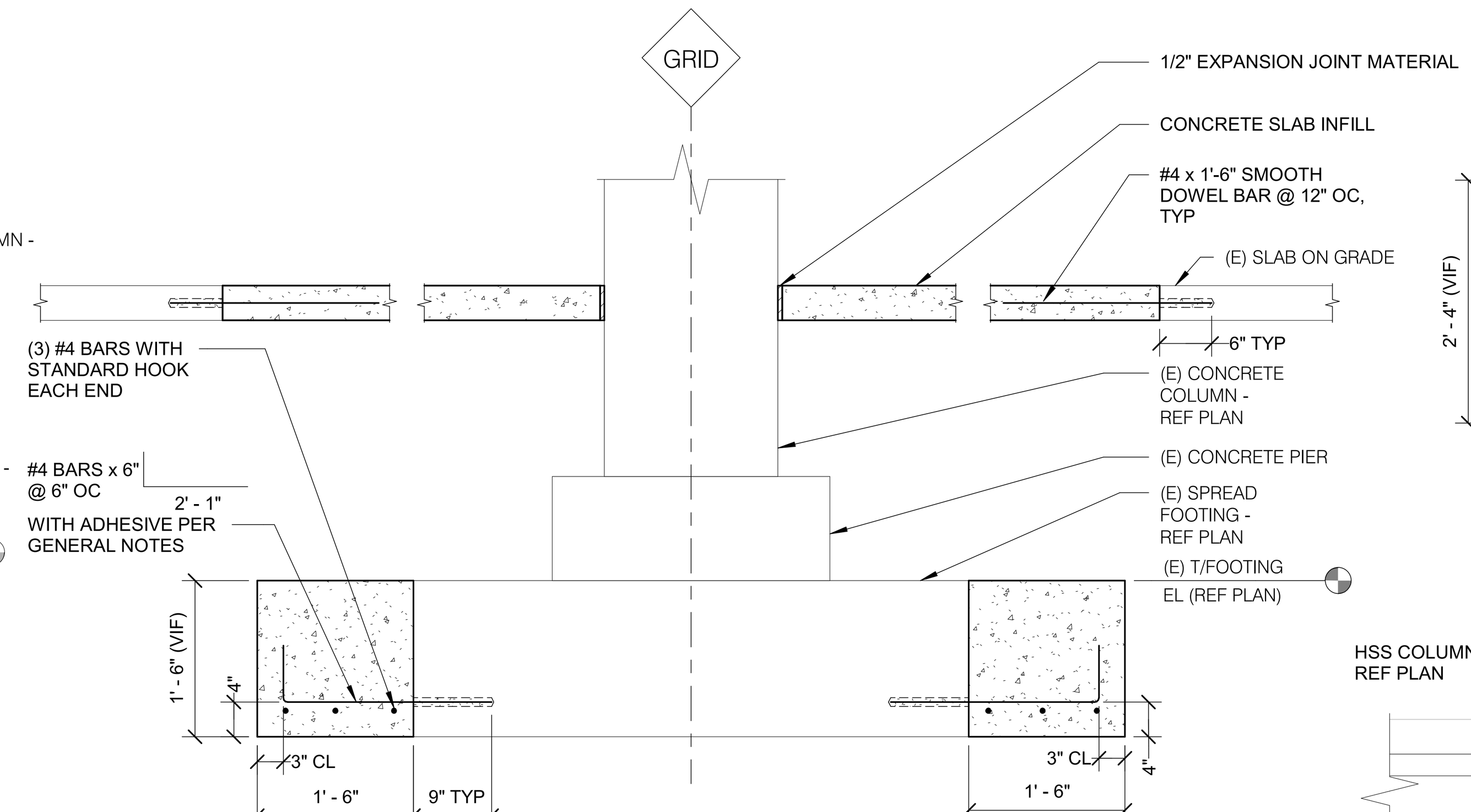


9 SPREAD FOOTING DETAIL AT EXISTING

3/4" = 1'-0"

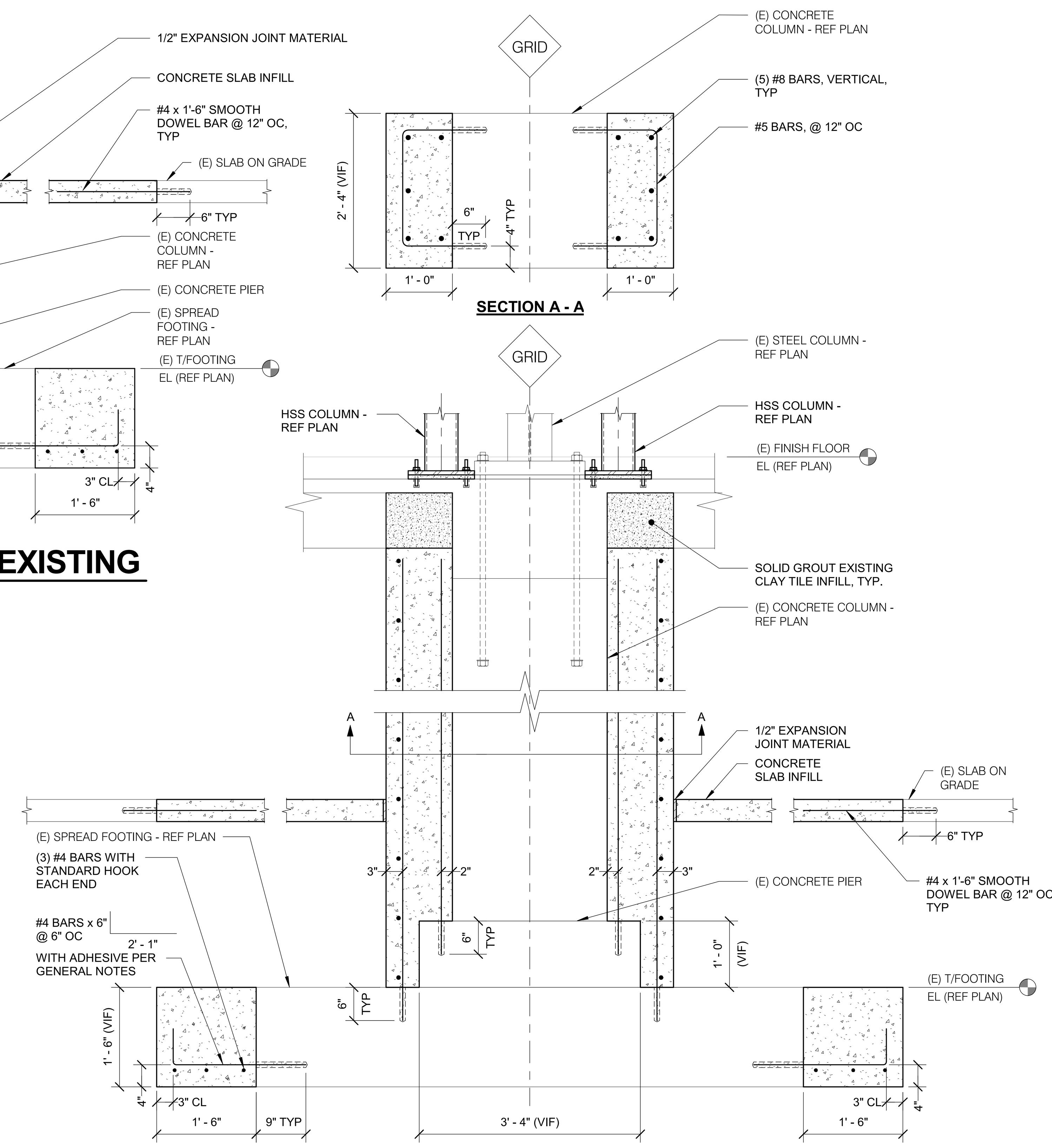
NOTES:

1. ONLY MODIFY EXISTING PIER AT SIM.



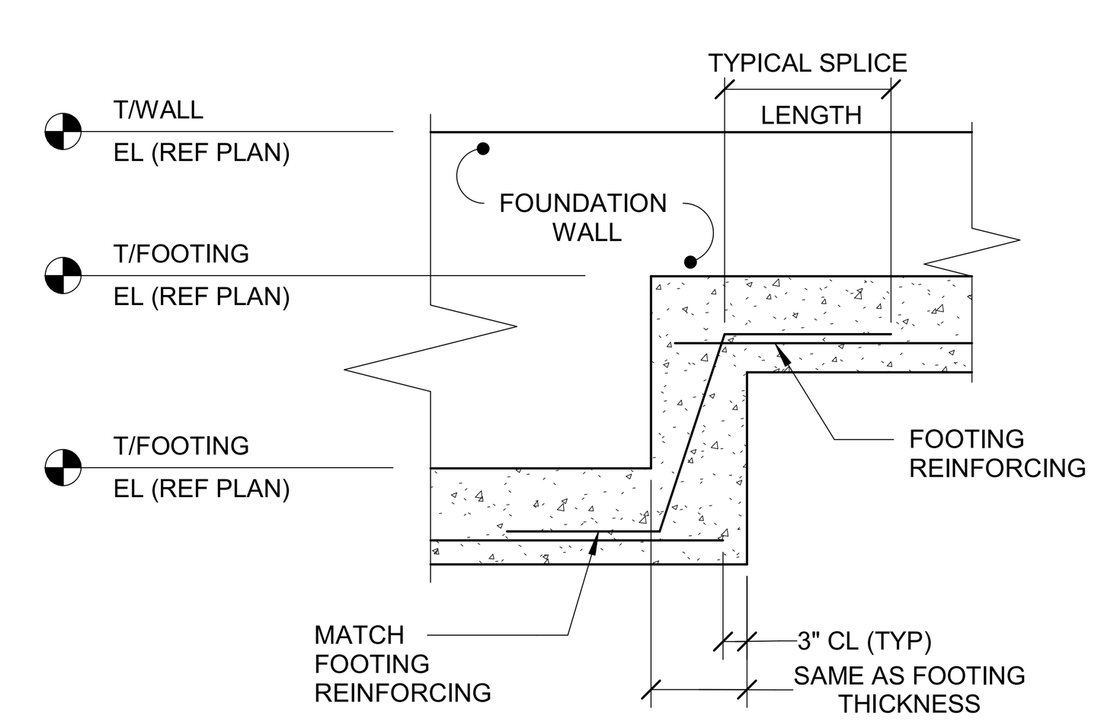
10 SPREAD FOOTING AT EXISTING

3/4" = 1'-0"



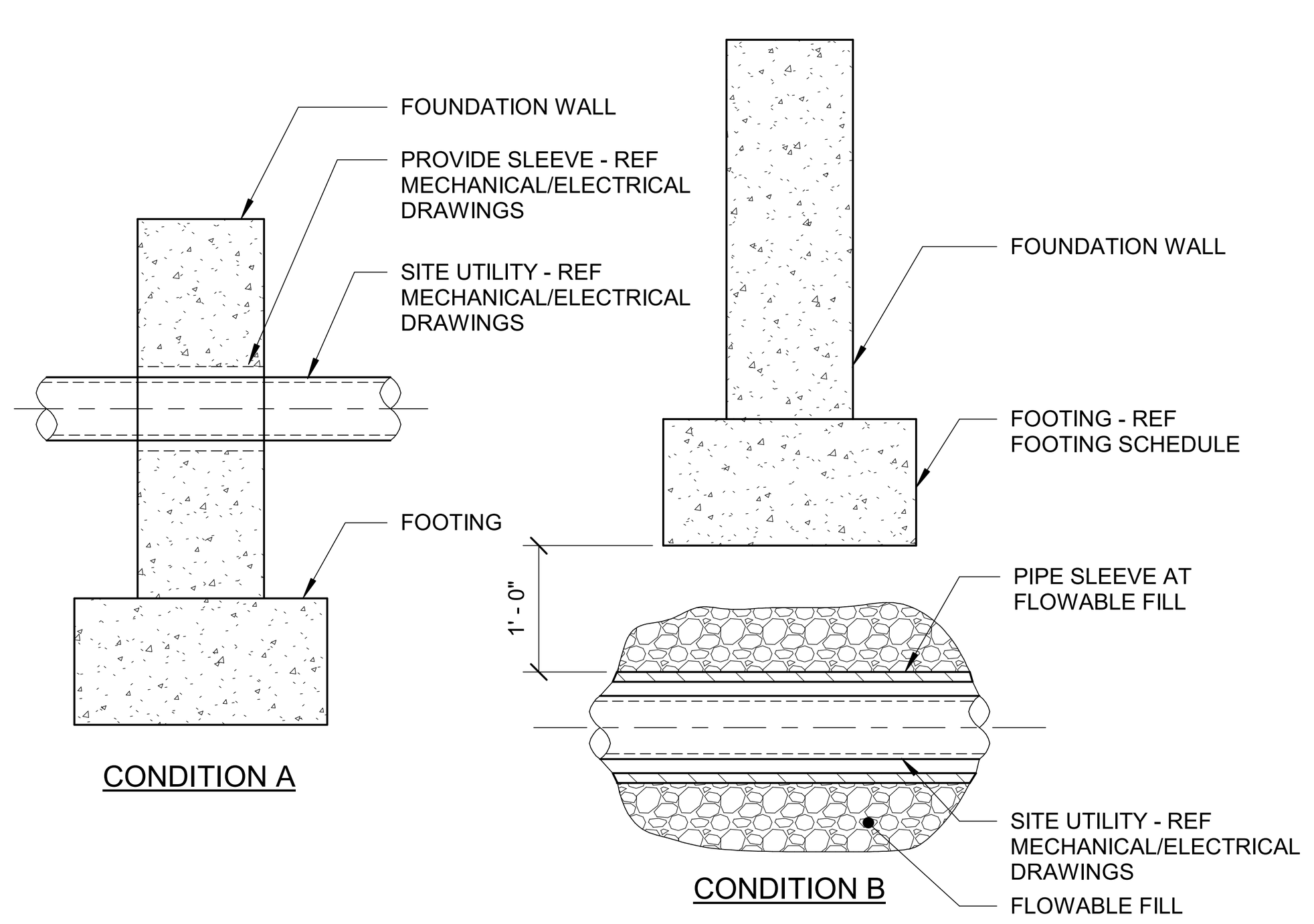
11 COLUMN AT EXISTING

3/4" = 1'-0"



12 TYPICAL FOOTING STEP

1/2" = 1'-0"



13 TYPICAL FOUNDATION DETAILS AT SITE UTILITIES

3/4" = 1'-0"

NOTES:

1. REFERENCE MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL LOCATIONS, ELEVATIONS, ETC., OF SITE UTILITIES.
2. DETAIL REQUIRED AT ALL UTILITIES HAVING A PLAN WIDTH UP TO 3'-0". FOR WIDTHS GREATER THAN 3'-0", REFERENCE PLAN FOR REQUIRED DETAIL.
3. CONDITION B DOES NOT APPLY AT SPREAD FOOTING SITUATIONS. GENERAL CONTRACTOR SHALL NOTIFY ENGINEER SHOULD SUCH A CONDITION ARISE AND AWAIT FURTHER INSTRUCTIONS.

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FOUNDATION DETAILS

EXHIBIT H

S201

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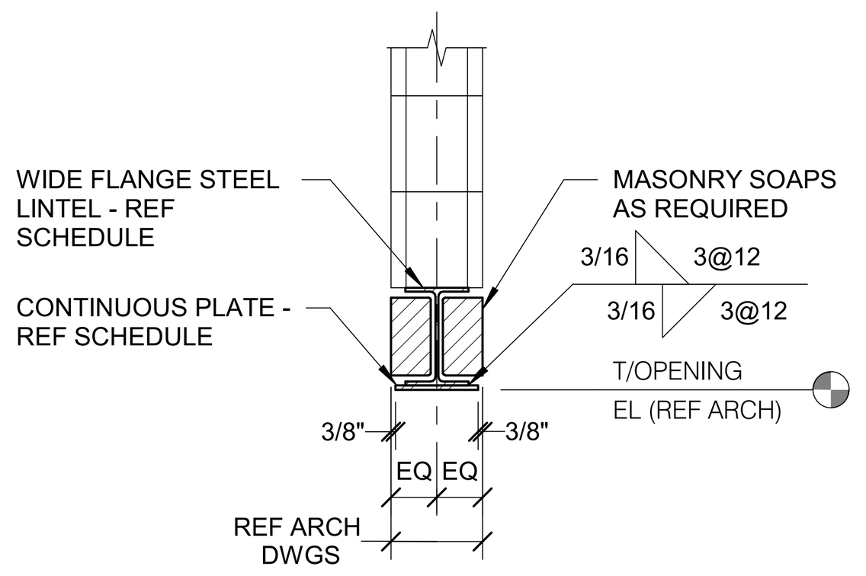
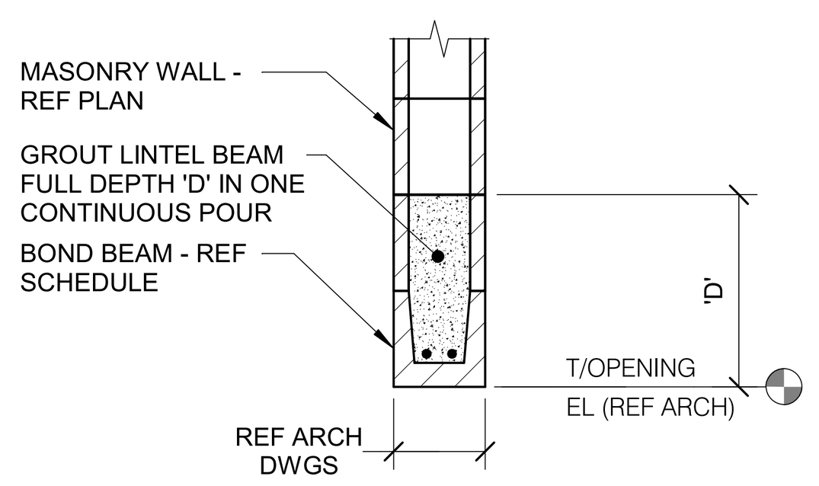


1 LINTEL DETAIL
3/4" = 1'-0"

NOTES:

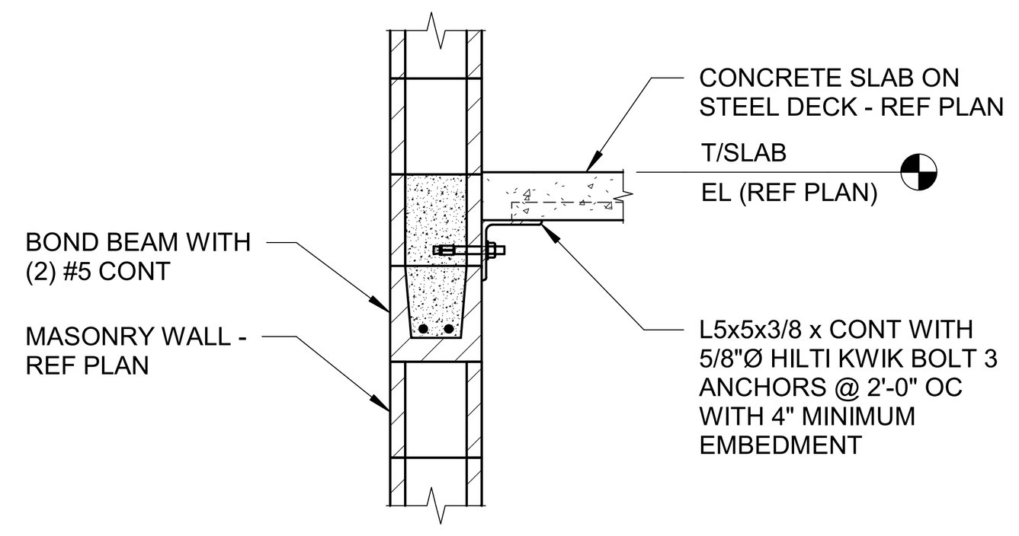
1. EXISTING MASONRY WALL TO BE SHORED AS REQUIRED FOR INSTALLATION OF NEW LINTEL.



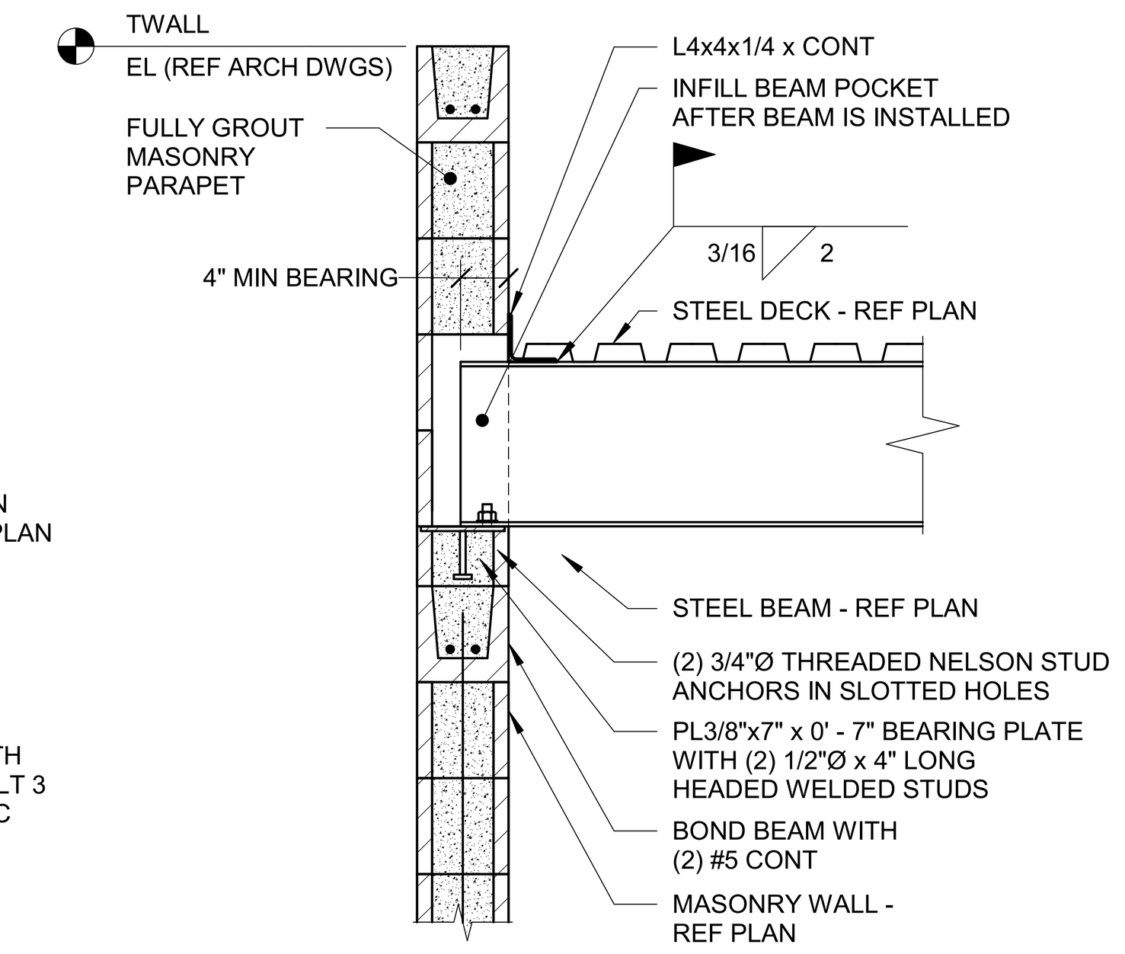
2 LINTEL DETAIL
3/4" = 1'-0"

NOTES:

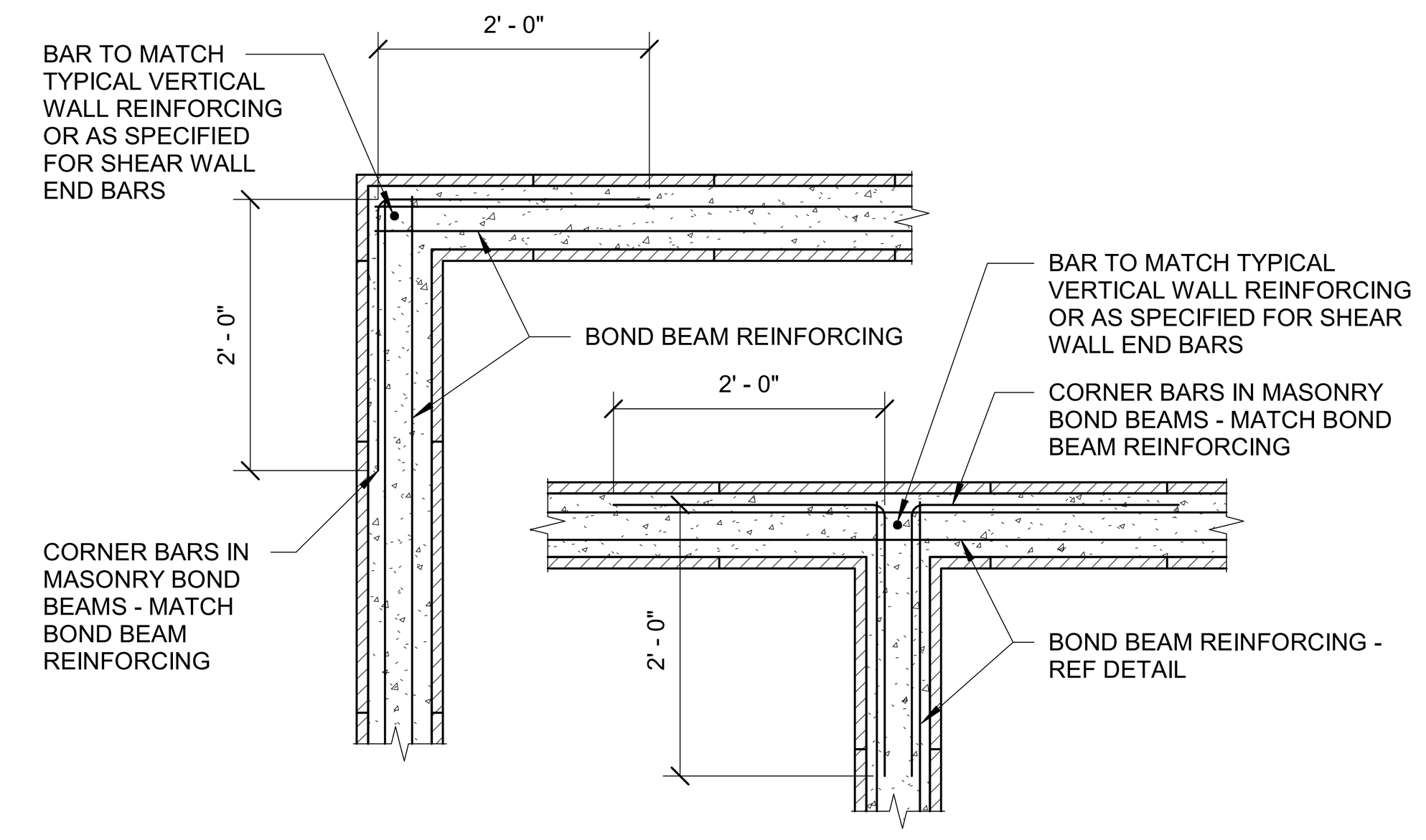
1. SHORE MASONRY UNTIL GROUT FOR LINTEL HAS REACHED ITS SPECIFIED STRENGTH.



3 TYPICAL DECK SUPPORT AT MASONRY
3/4" = 1'-0"



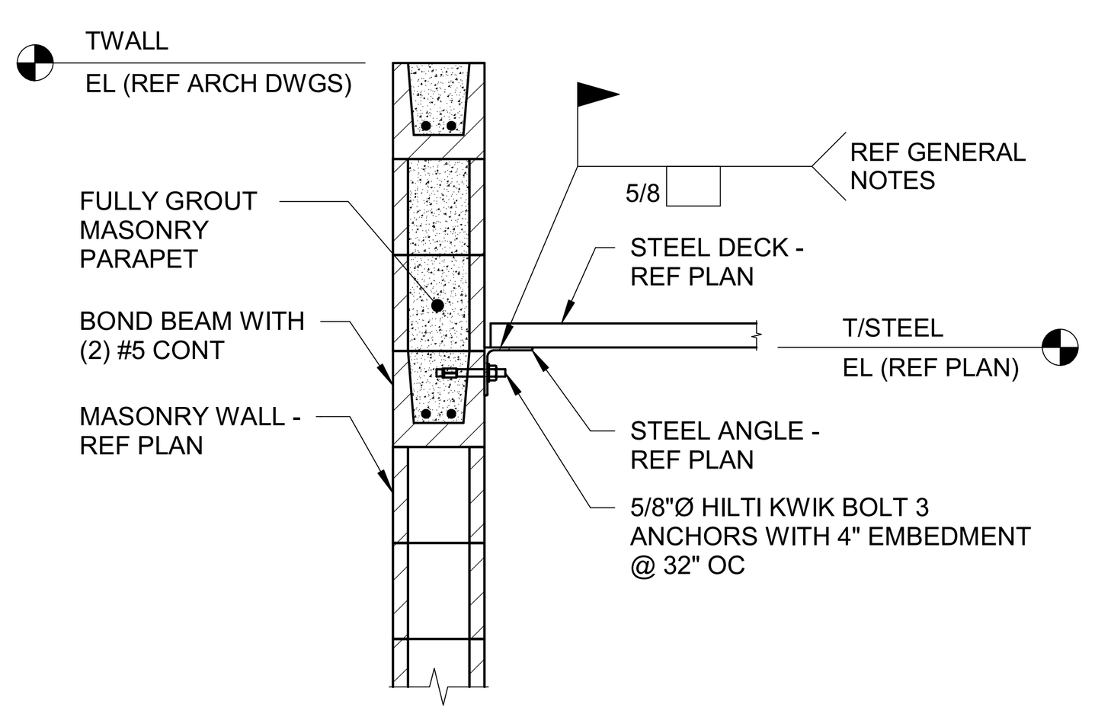
4 TYPICAL BEAM BEARING ON MASONRY
3/4" = 1'-0"



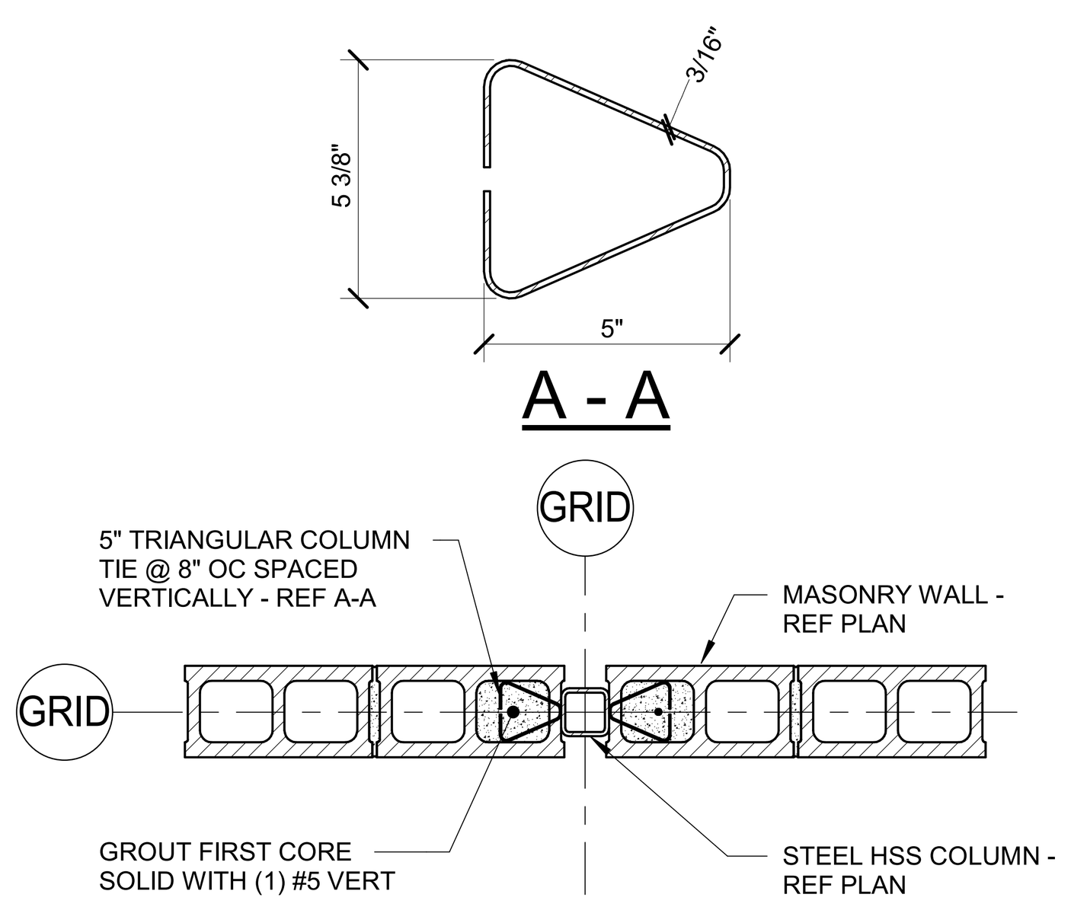
5 TYPICAL CMU WALL CORNER REINFORCING
3/4" = 1'-0"

NOTES:

1. TYPICAL VERTICAL WALL REINFORCING NOT SHOWN FOR CLARITY.



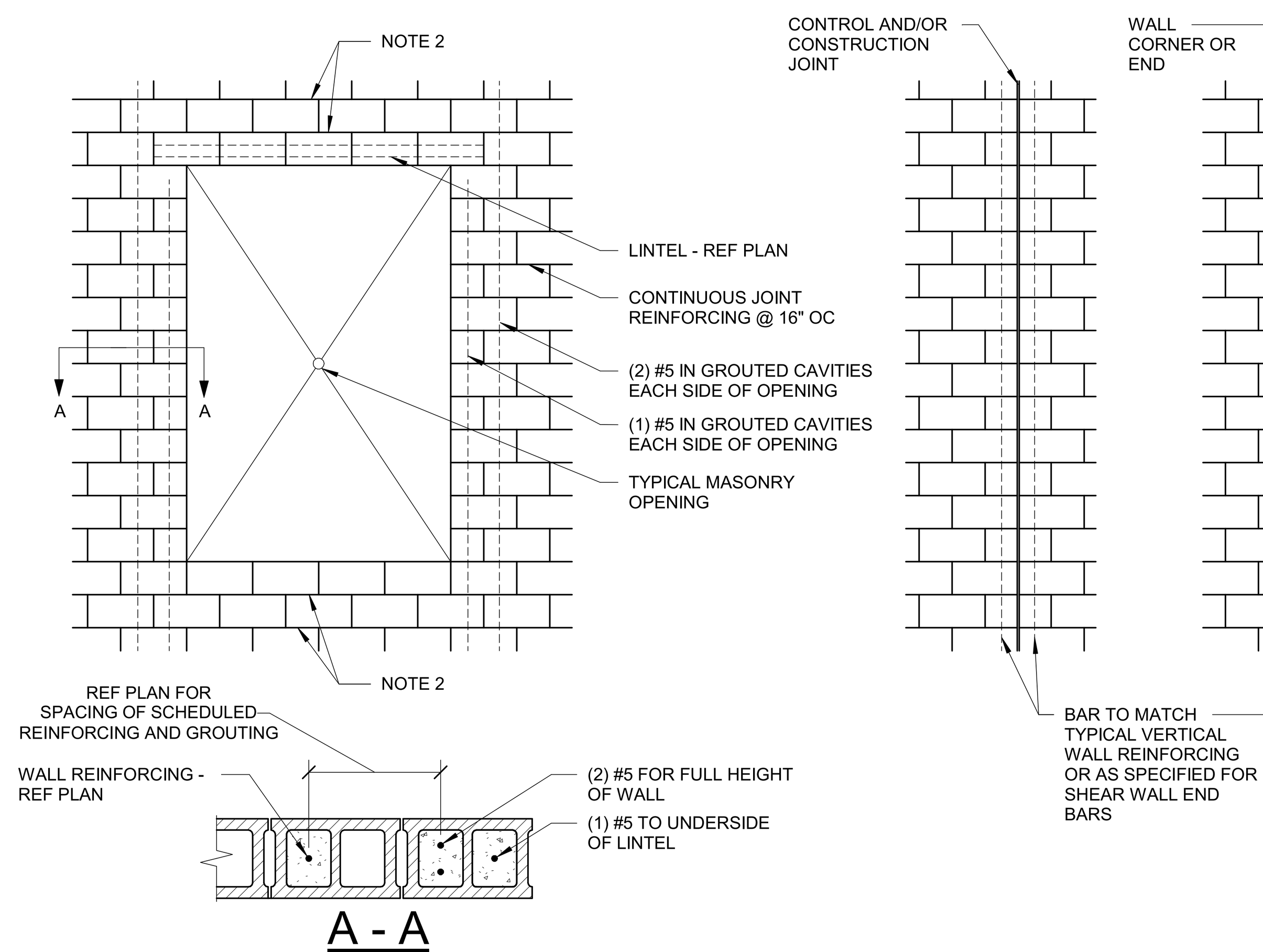
6 ROOF DECK SUPPORT AT MASONRY
3/4" = 1'-0"



7 COLUMN BRACING DETAIL AT CMU WALLS
3/4" = 1'-0"

NOTES:

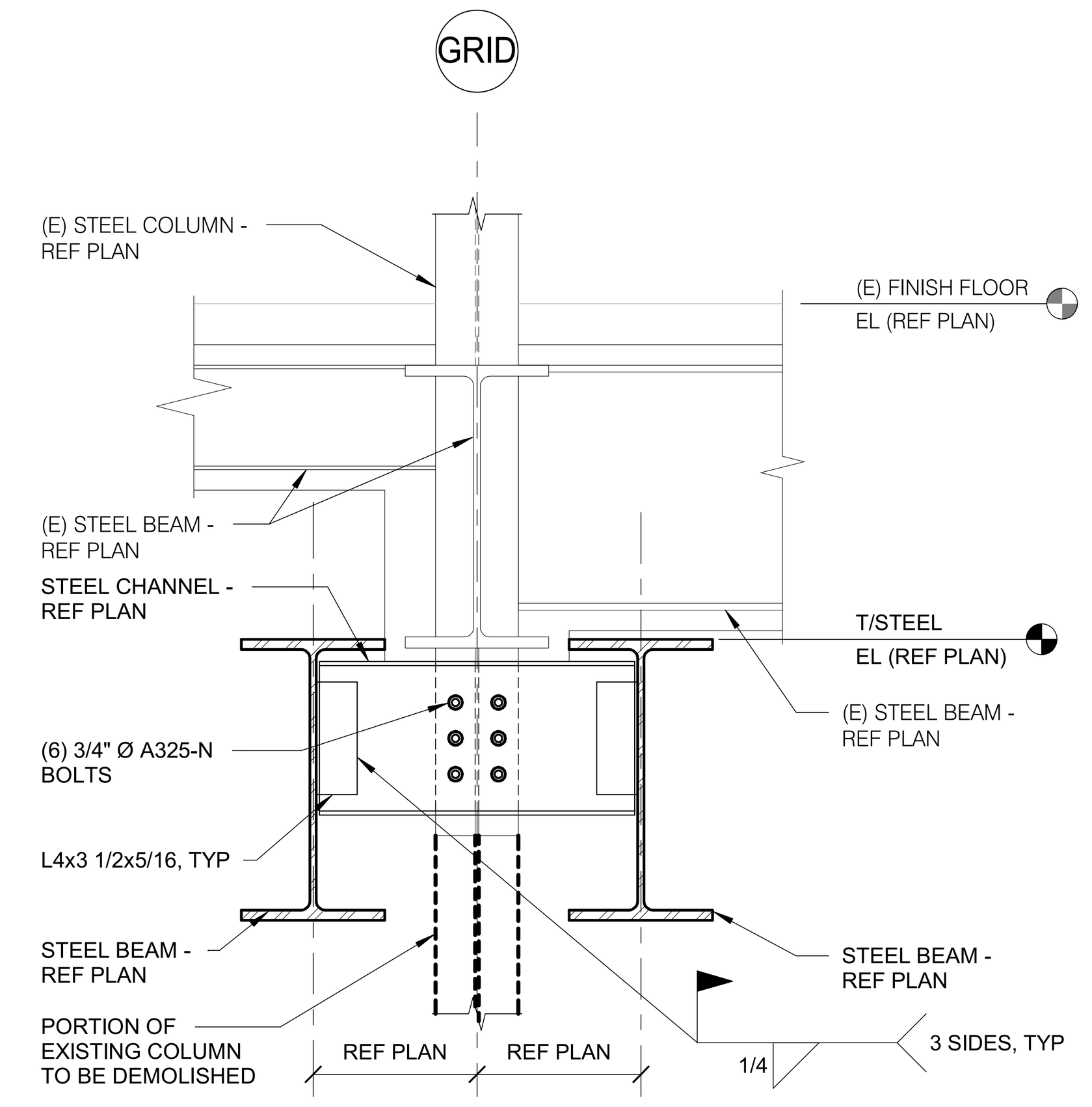
1. DETAIL TO BE SIMILAR FOR WIDE FLANGE COLUMNS.



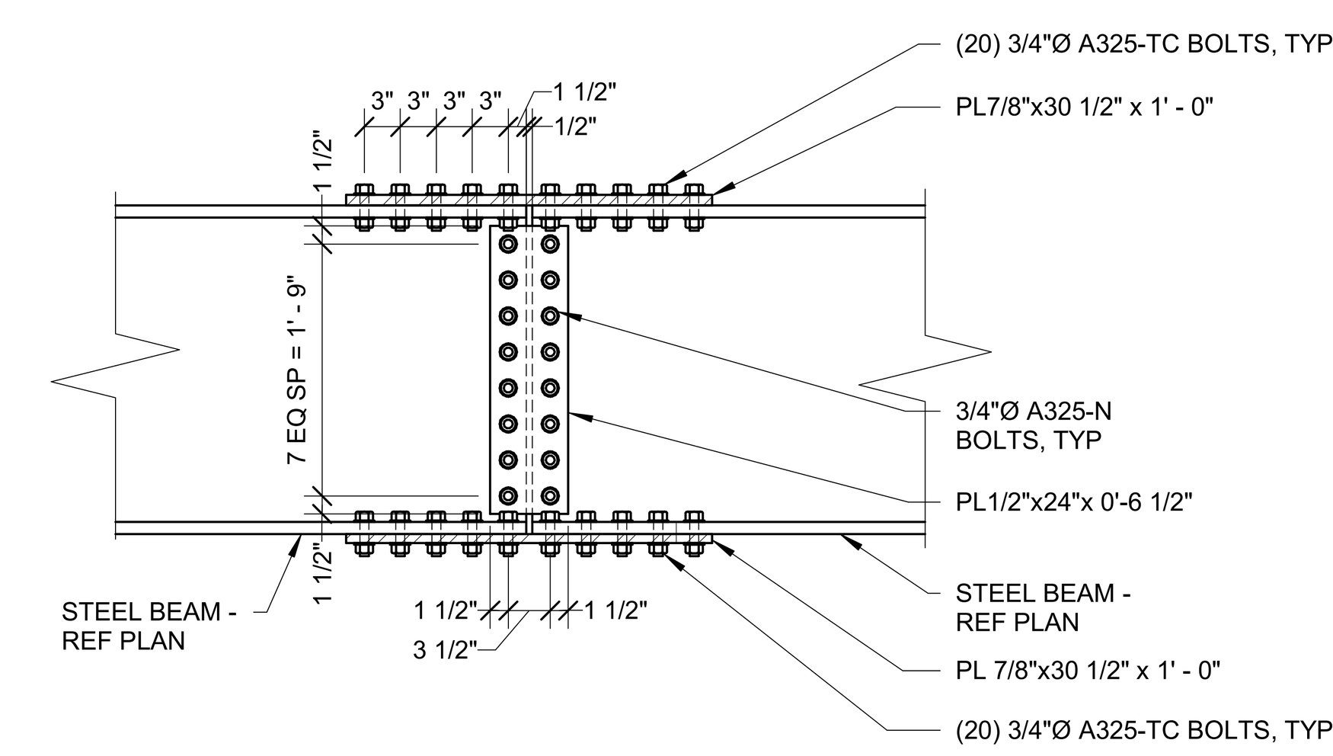
8 TYPICAL MASONRY WALL DETAIL
3/4" = 1'-0"

NOTES:

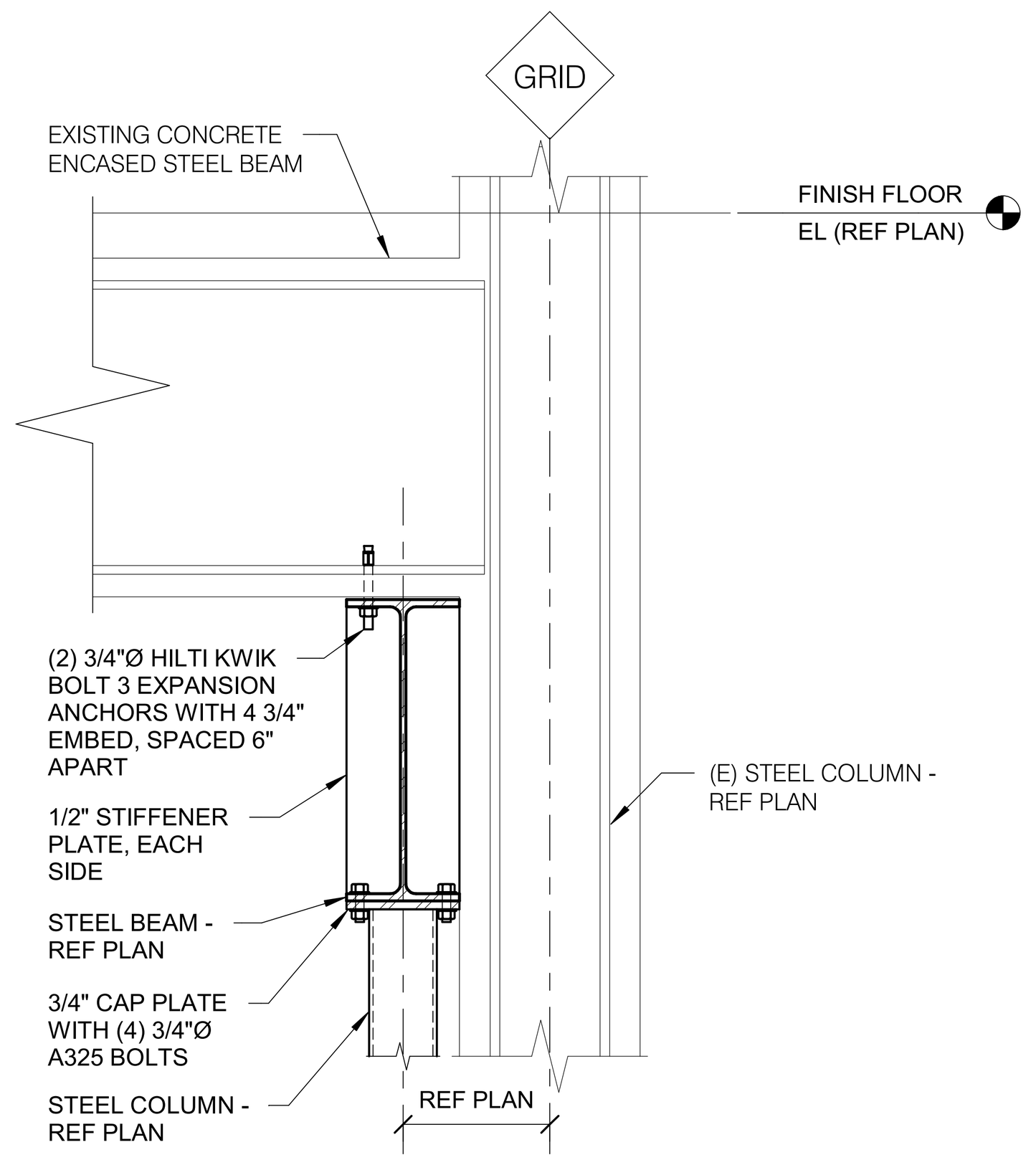
1. REFER TO ARCHITECTURAL ELEVATIONS FOR MASONRY CONTROL JOINT LOCATIONS.
2. TWO COURSES OF JOINT REINFORCING ARE REQUIRED ABOVE THE LINTEL AND BELOW THE SILL AND SHALL EXTEND A MINIMUM OF 24 INCHES PAST THE OPENING.



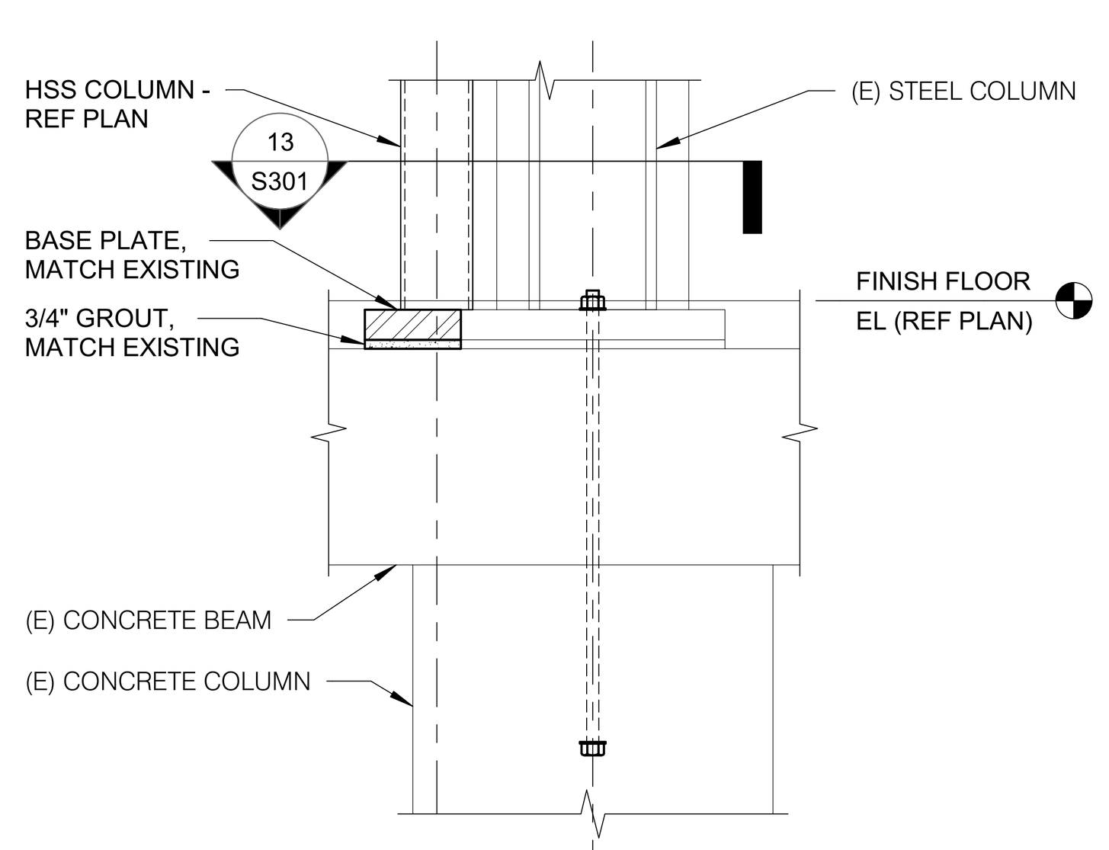
9 BEAM COLUMN CONNECTION
3/4" = 1'-0"



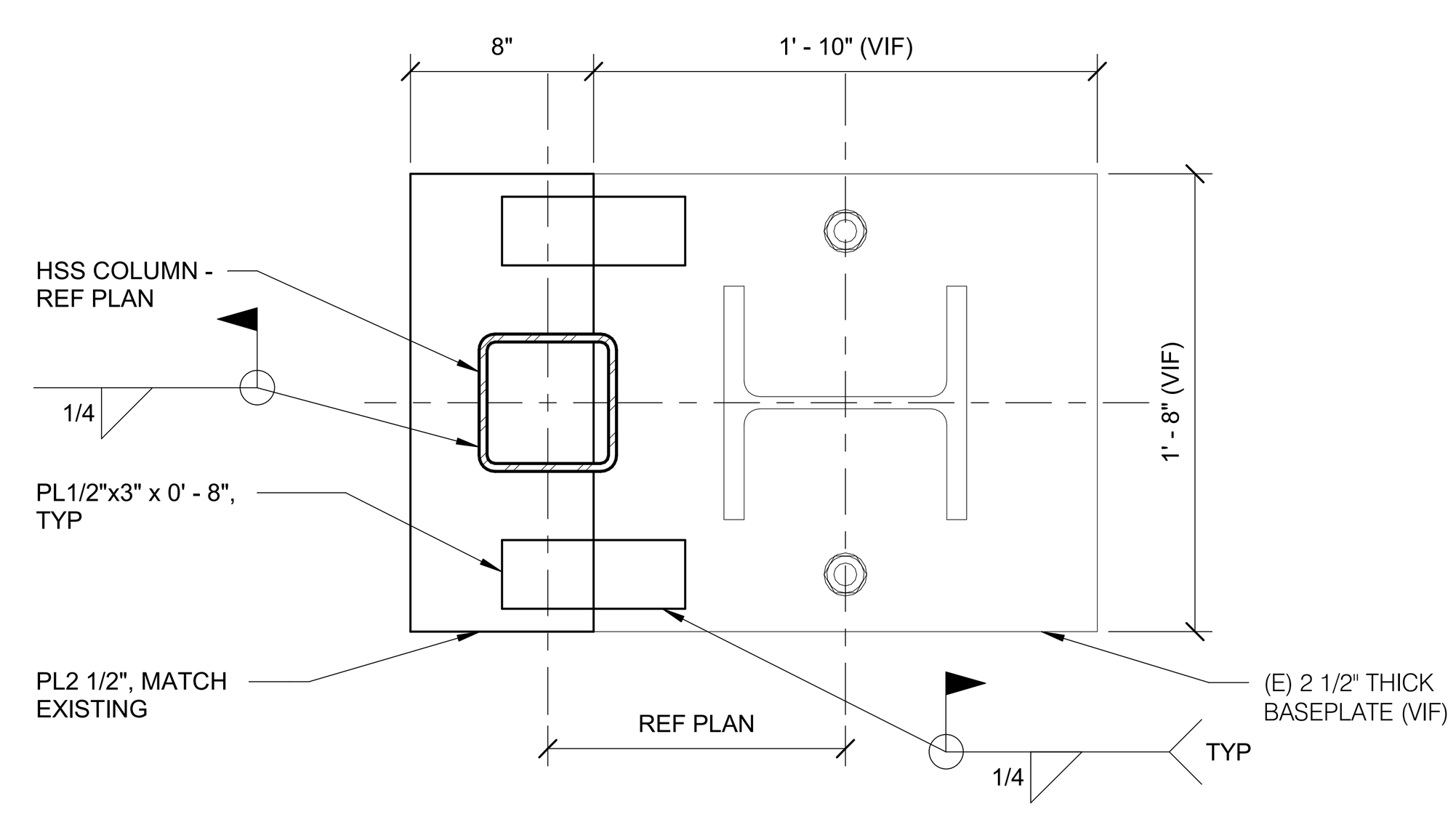
10 TYPICAL BOLTED SPLICE DETAIL
3/4" = 1'-0"



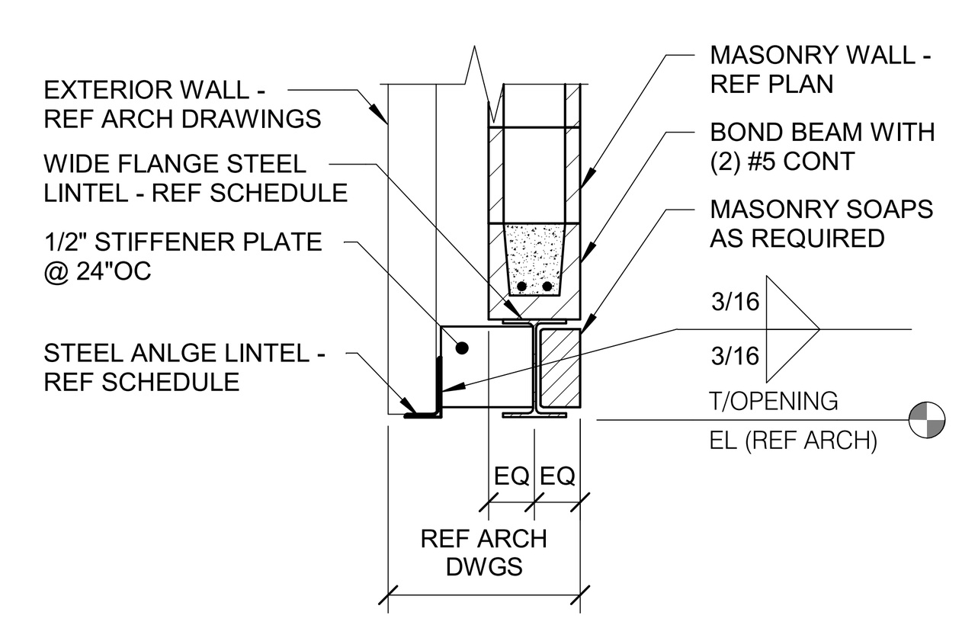
11 COLUMN AT EXISTING
3/4" = 1'-0"



12 BASE PLATE (BP5) DETAIL
3/4" = 1'-0"



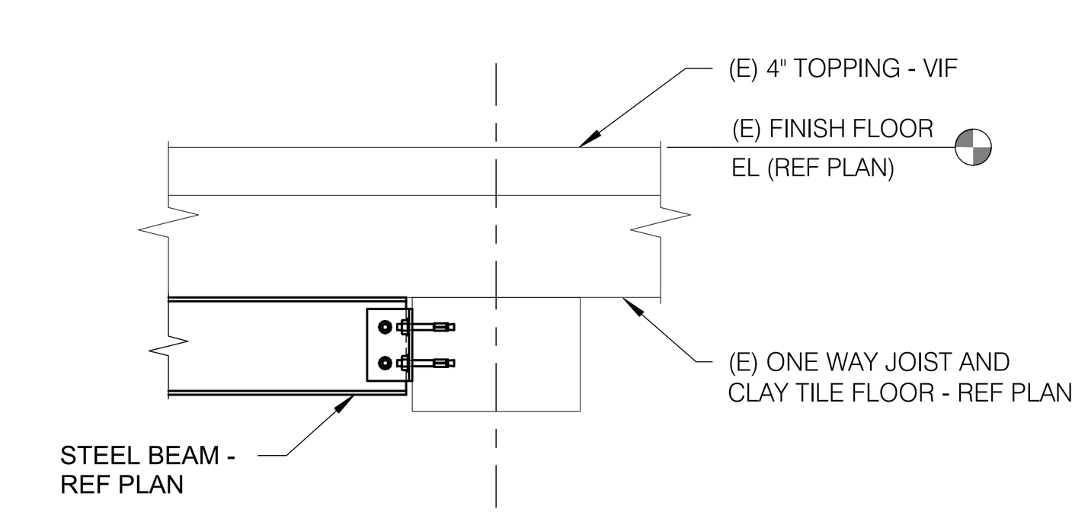
13 BASE PLATE (BP5) DETAIL
1 1/2" = 1'-0"



14 LINTEL DETAIL
3/4" = 1'-0"

NOTES:

1. REFERENCE ARCHITECTURAL DRAWINGS FOR INSULATION, THROUGH-WALL FLASHING, AND WEEP HOLES.
2. PROVIDE 3/8" BOTTOM PLATE AT STEEL BEAM AS REQUIRED TO CARRY MASONRY SOAPS.



15 FRAMING DETAIL AT EXISTING
3/4" = 1'-0"

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Civil Engineers
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Madison Municipal Building Renovation

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I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the Laws of the State of Wisconsin.
ARCHITECT SEAL

Signature:
Print Names:
Date: License No:

ISSUE

MARK	DATE	DESCRIPTION
	03.24.2017	BID SET

PROJECT NO. 2014057
PROJECT PHASE BID SET
DRAWN BY: PRIPAN
CHECKED BY: ABBPER

KJWW ENGINEERING CONSULTANTS
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MIDDLETON, WISCONSIN 53562
608.223.9600 FAX: 608.836.0415
www.kjww.com
PROJECT # 00140628.00

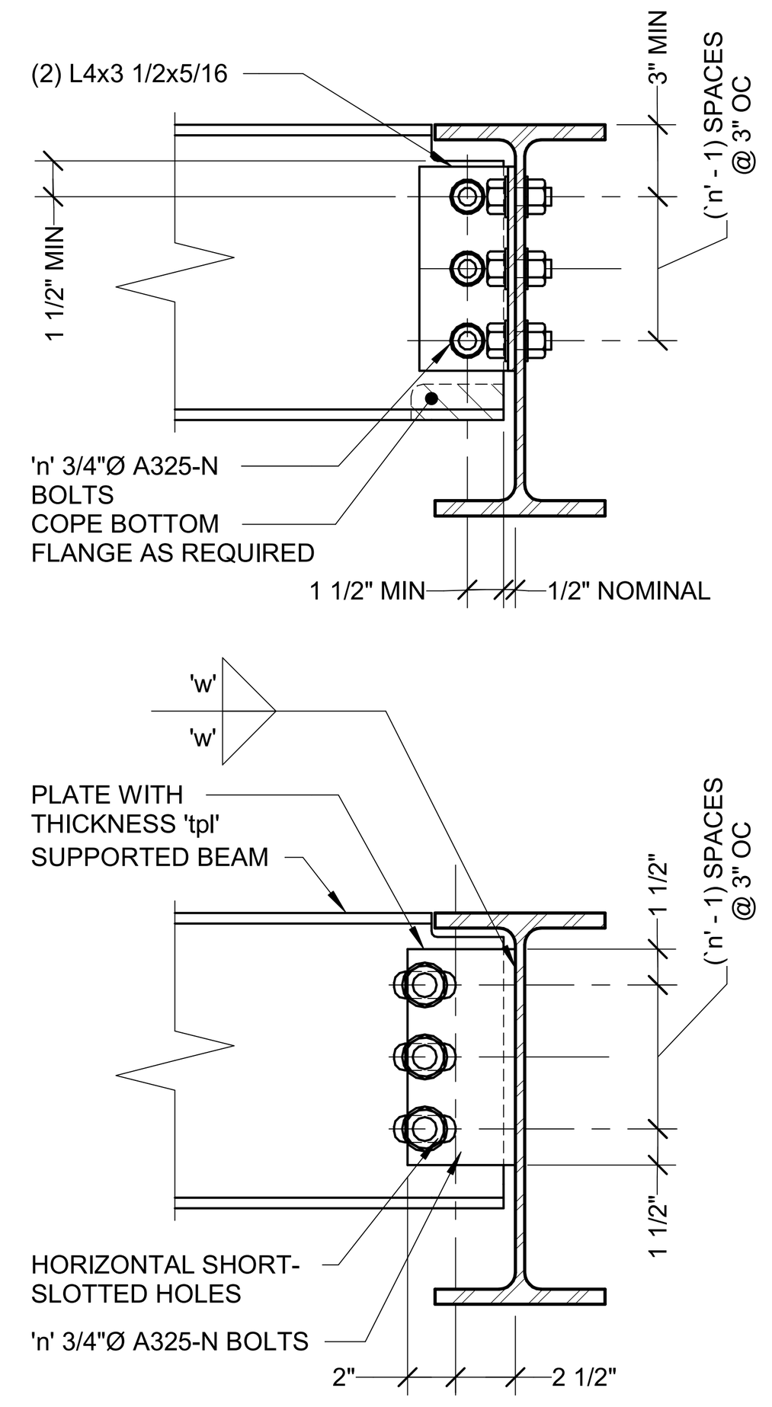
REFERENCE SCALE IN INCHES
0 1 2 3

FRAMING DETAILS

EXHIBIT H

S301

NOT FOR CONSTRUCTION



DOUBLE ANGLE CONNECTION TABLE	
SUPPORTED BEAM	**'n'**
W8, W10	2
W12, W14	3
W16	4
W18	5
W21	6
W24, W27	7
W30	8
W33	9
W36	10
W40	11
W44	12

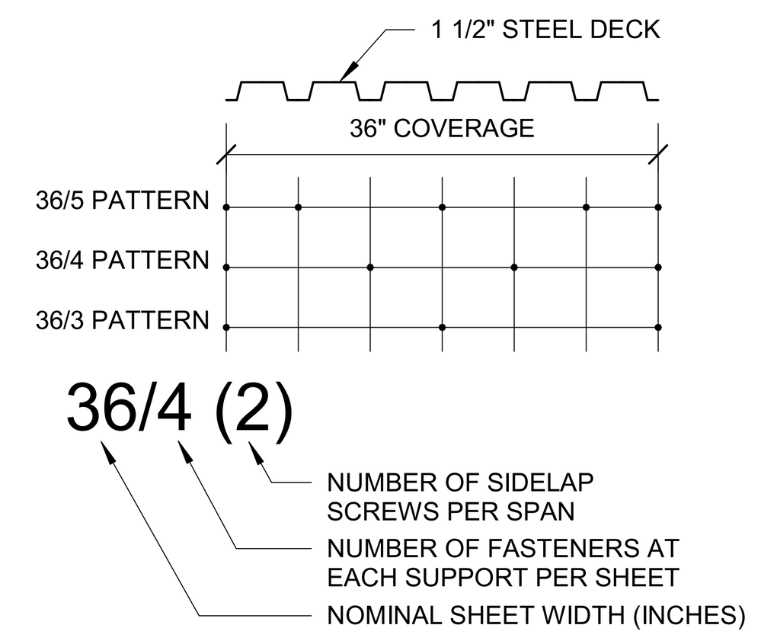
KNIFE PLATE CONNECTION TABLE			
SUPPORTED BEAM	**'tpl'**	**'n'**	**'w'**
W8, W10	1/4"	2	3/16"
W12, W14	5/16"	3	1/4"
W16	5/16"	4	1/4"
W18	5/16"	5	1/4"
W21	5/16"	6	1/4"
W24, W27	3/8"	7	1/4"
W30	3/8"	8	1/4"
W33	3/8"	9	1/4"
W36	3/8"	10	1/4"
W40	3/8"	11	1/4"
W44	3/8"	12	1/4"

1 TYPICAL SHEAR CONNECTION

1 1/2" = 1'-0"

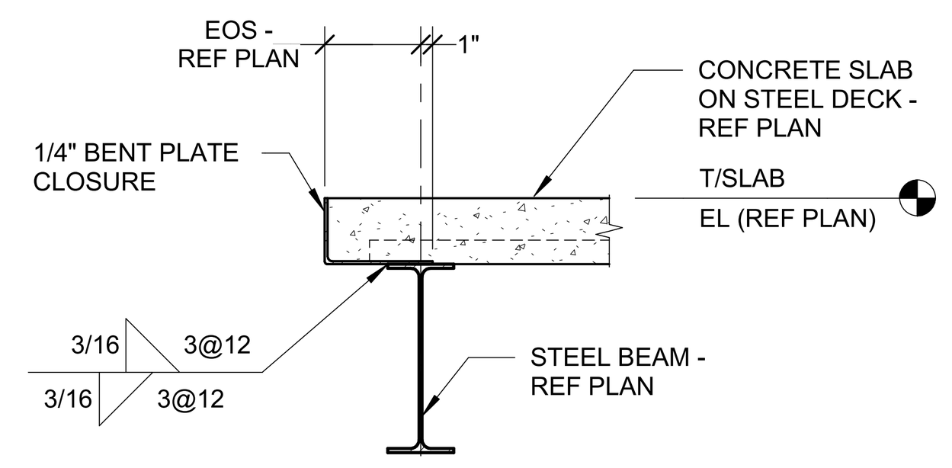
NOTES:

1. BOTH DOUBLE ANGLE AND KNIFE PLATE CONNECTION CONFIGURATIONS ARE ACCEPTABLE, UNLESS NOTED OTHERWISE. FABRICATOR AND DETAILER SHALL SELECT WHICH OPTION IS BEST SUITED FOR THEIR FABRICATION PROCESS AND THE ANTICIPATED ERECTION PROCEDURES.
2. DETAIL TO BE SIMILAR AT CONNECTIONS TO WIDE FLANGE OR HSS COLUMNS.
3. UNLESS NOTED OTHERWISE, PROVIDE SHEAR CONNECTIONS AS INDICATED BY THIS DETAIL.
4. DETAILER IS RESPONSIBLE FOR FULLY DEVELOPING GEOMETRY AND DIMENSIONAL INFORMATION REQUIRED TO FABRICATE.
5. WHERE TYPICAL SHEAR CONNECTION DETAIL IS NOT APPLICABLE, FABRICATOR SHALL SELECT AND DETAIL ALTERNATE CONNECTION CAPABLE OF DEVELOPING EQUAL STRENGTH. ALTERNATE CONNECTION SHALL BE SELECTED IN ACCORDANCE WITH AISC ASD CONNECTION TABLES.



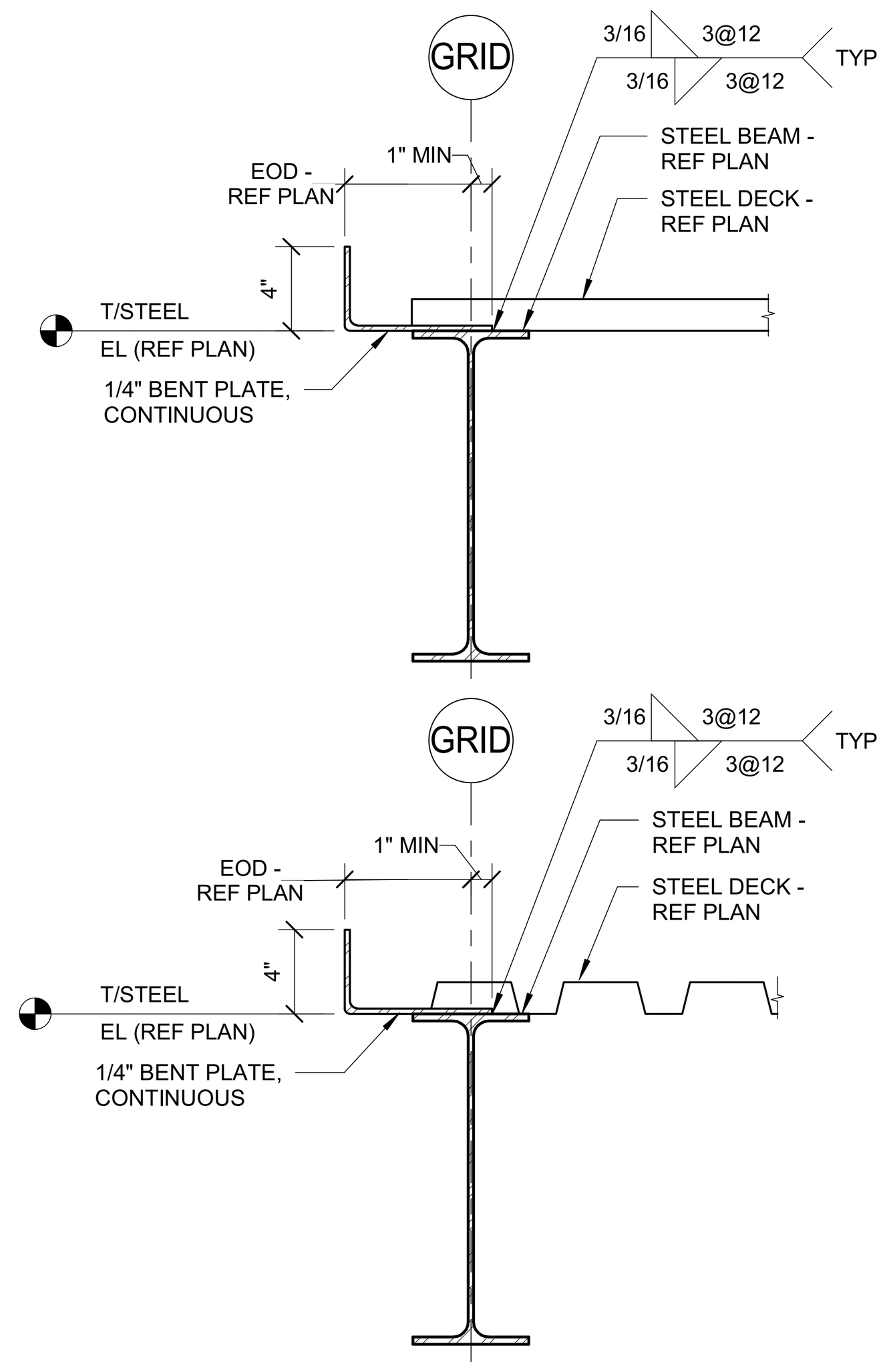
2 DECK FASTENER LAYOUT

3/4" = 1'-0"



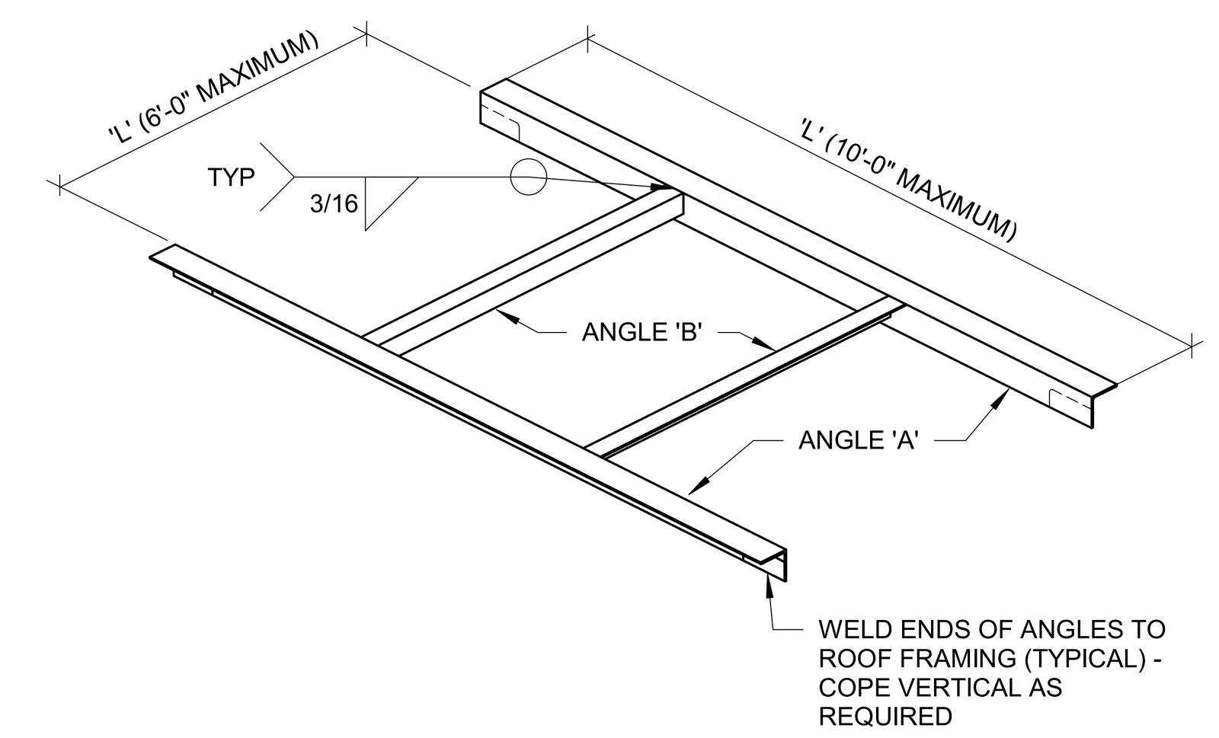
3 TYPICAL SLAB EDGE DETAIL

3/4" = 1'-0"



4 TYPICAL CLOSURE PLATE DETAIL

1 1/2" = 1'-0"



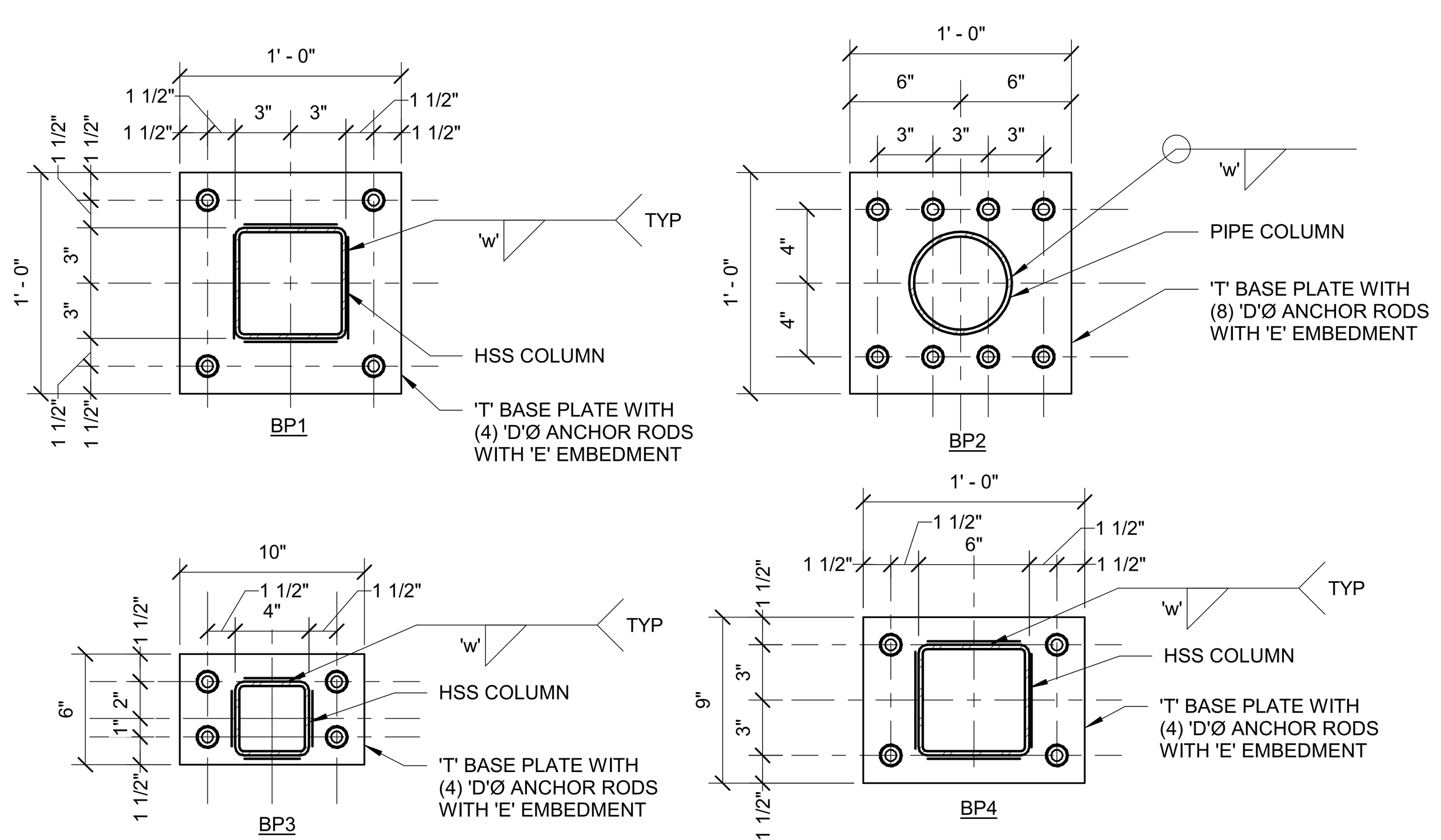
'L'	ANGLE 'A'	ANGLE 'B'
UP TO 1'-0"	NONE - SUMP PAN ONLY	NONE - SUMP PAN ONLY
1'-1" TO 4'-6"	L4x4x1/4	L4x4x1/4
4'-7" TO 6'-0"	L4x4x5/16	L4x4x1/4
6'-1" TO 8'-0"	L4x4x3/8	-
8'-1" TO 10'-0"	L6x4x3/8 (LLV)	-

5 DECK OPENING FRAMING DETAIL

3/4" = 1'-0"

NOTES:

1. USE ABOVE FRAMING AT ALL OPENINGS EXCEEDING 1'-0" UNO.
2. REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
3. ROOF OPENING FRAMING NOT REQUIRED AT SIDE DISCHARGE ROOF DRAINS. COORDINATE WITH MECHANICAL CONTRACTOR.



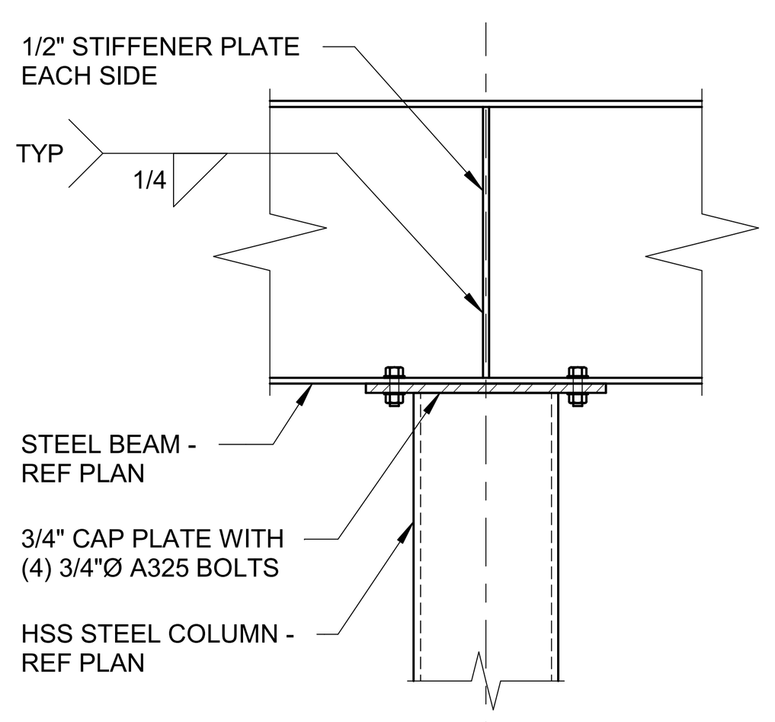
HSS BASE PLATE SCHEDULE						
MARK	**COLUMN**	**'T'**	**'D'**	**'E'**	**'w'**	**COMMENTS**
BP2	5STD	3/4"	5/8"	4"	1/4"	HILTI KWIK TZ
BP3	HSS4x4	3/4"	5/8"	4"	1/4"	HILTI KWIK TZ
BP4	HSS6x6	3/4"	3/4"	4"	5/16"	ADHESIVE ANCHOR - REF S000
BP5	HSS6x6	7/8"	3/4"	4"	5/16"	ADHESIVE ANCHOR - REF S000

9 BASE PLATE DETAIL

1 1/2" = 1'-0"

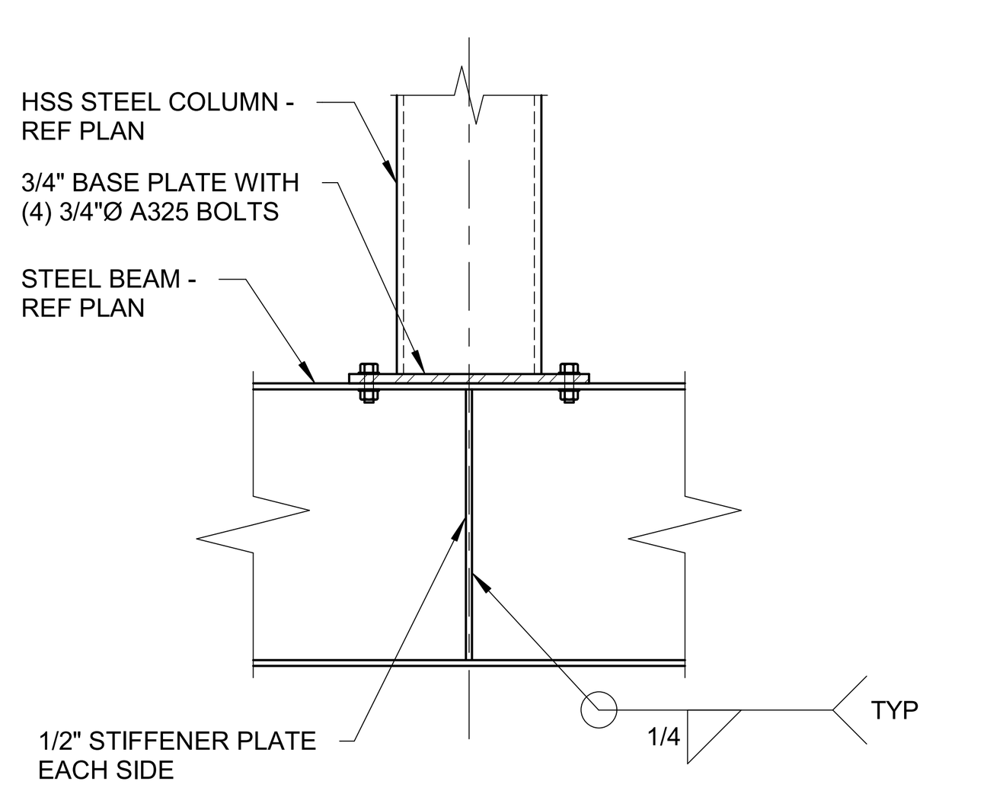
NOTES:

1. NO WELDS REQUIRED AT RADIUSES.



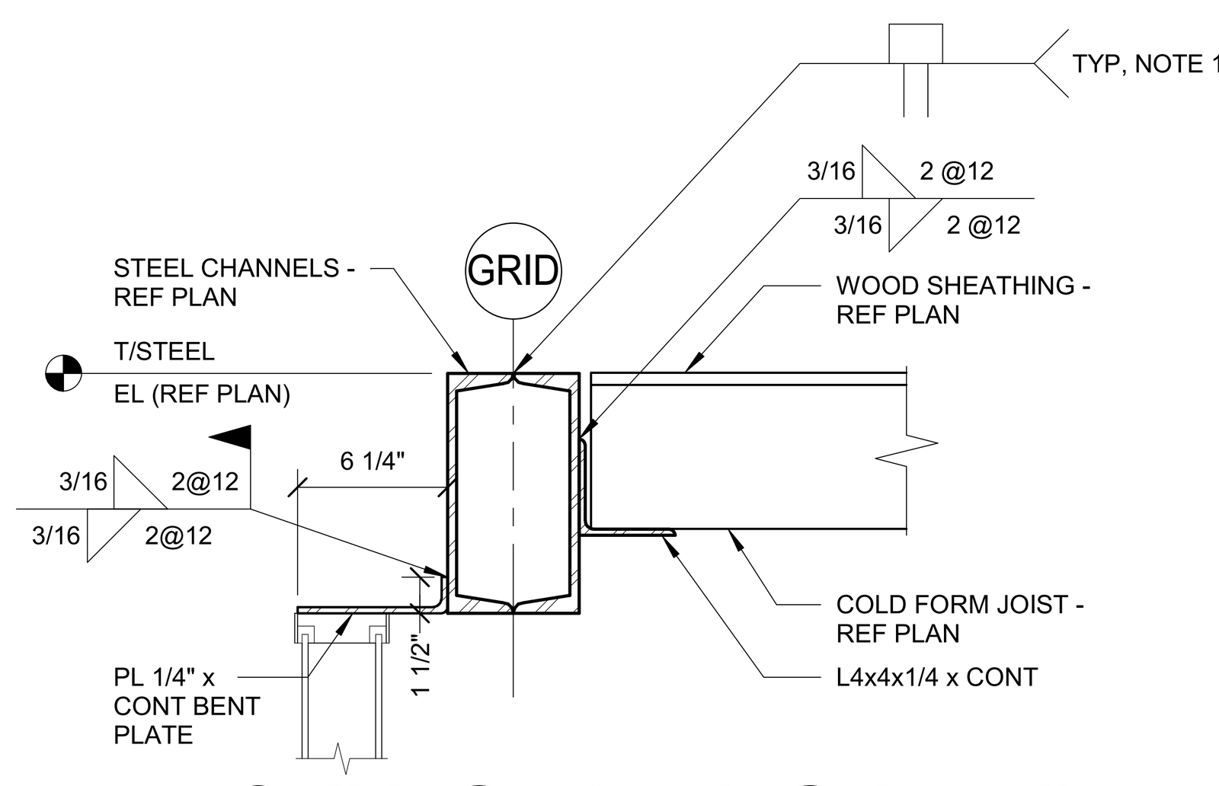
6 BEAM BEARING ON COLUMN

3/4" = 1'-0"



7 COLUMN BEARING ON BEAM

3/4" = 1'-0"

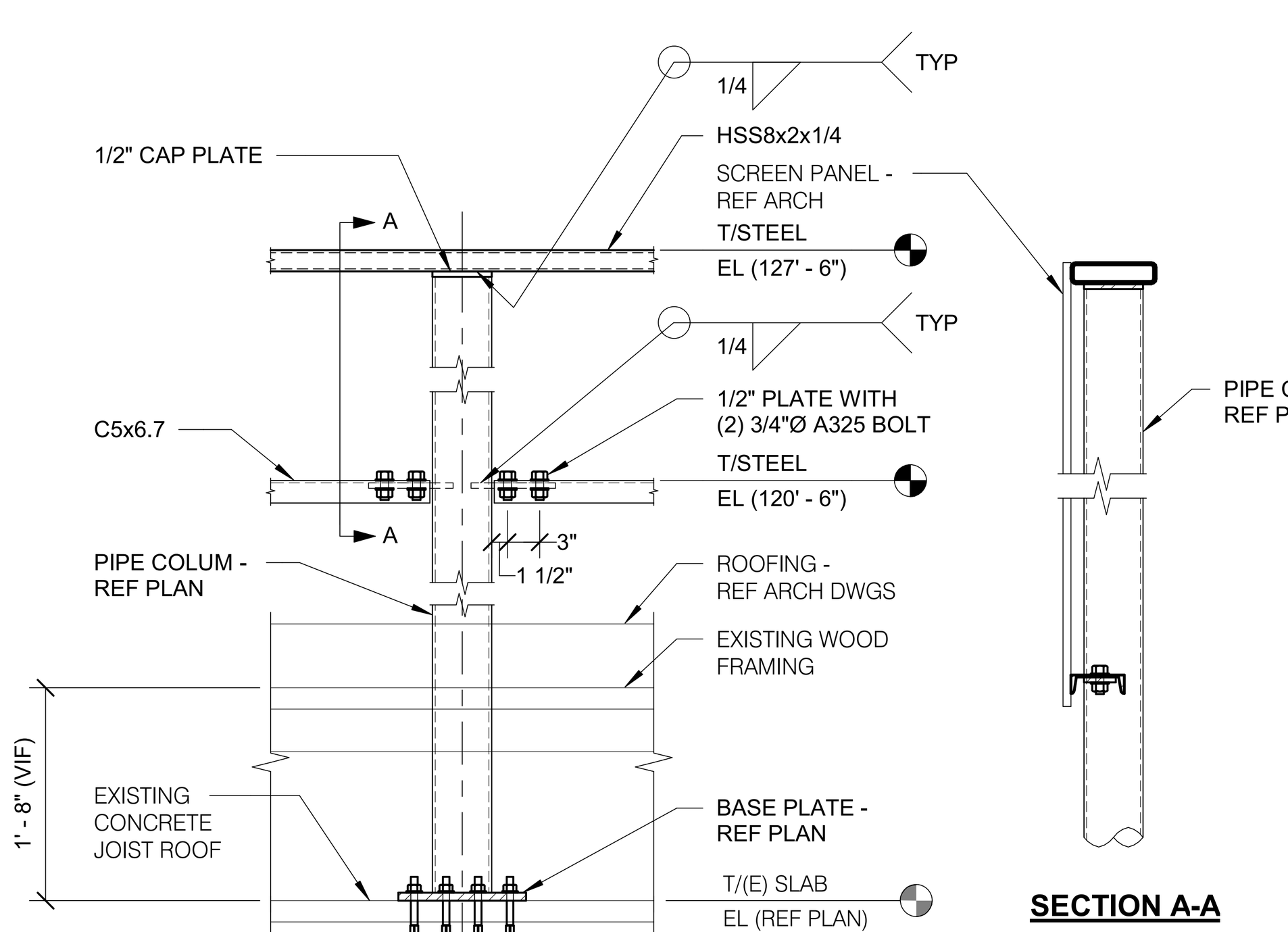


8 CEILING FRAMING DETAIL

1 1/2" = 1'-0"

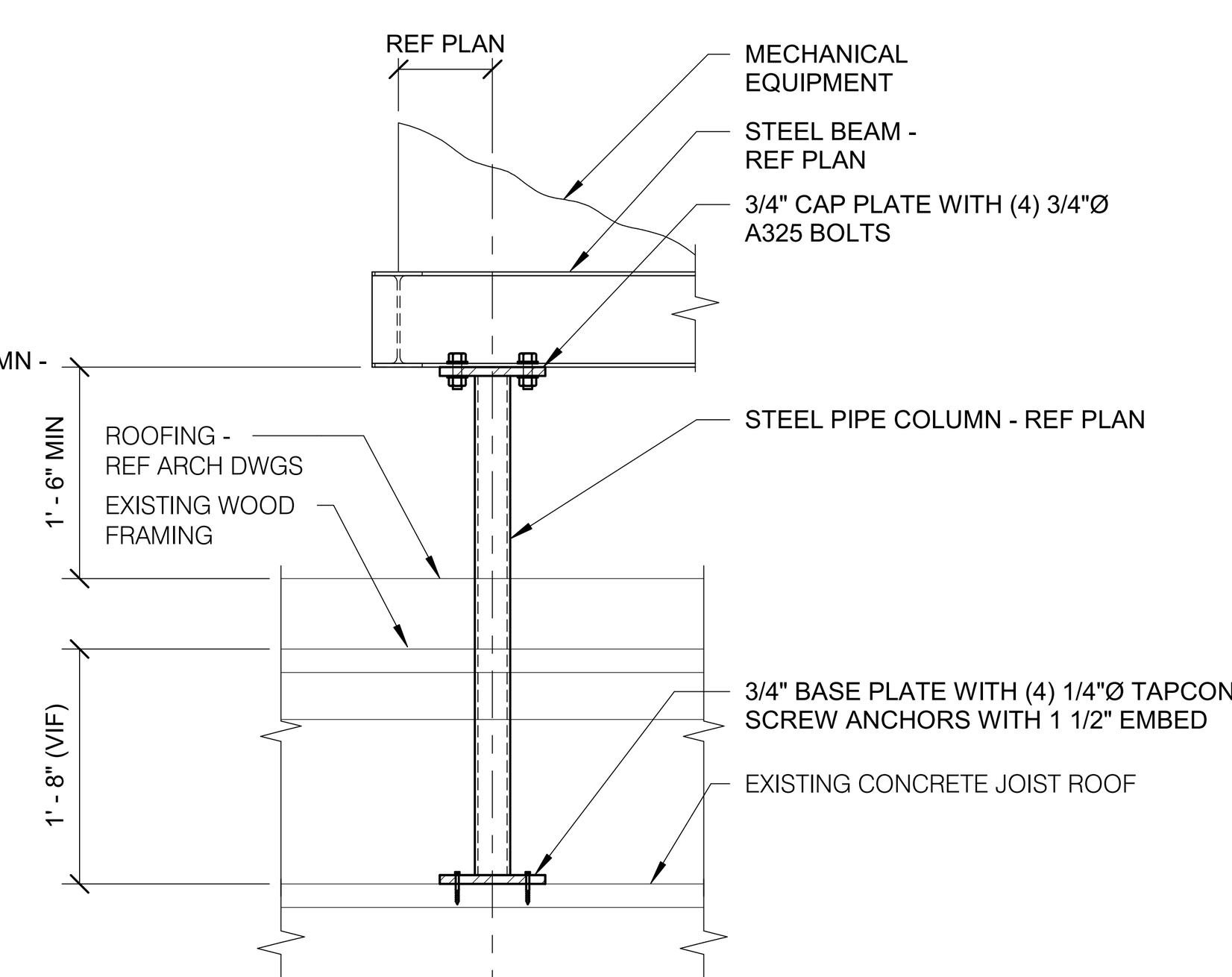
NOTES:

1. GRIND WELD SMOOTH ON BOTTOM SIDE, WHERE EXPOSED TO VIEW.
2. TOP OF WOOD SHEATHING TO ALIGN WITH TOP OF STEEL CHANNELS.



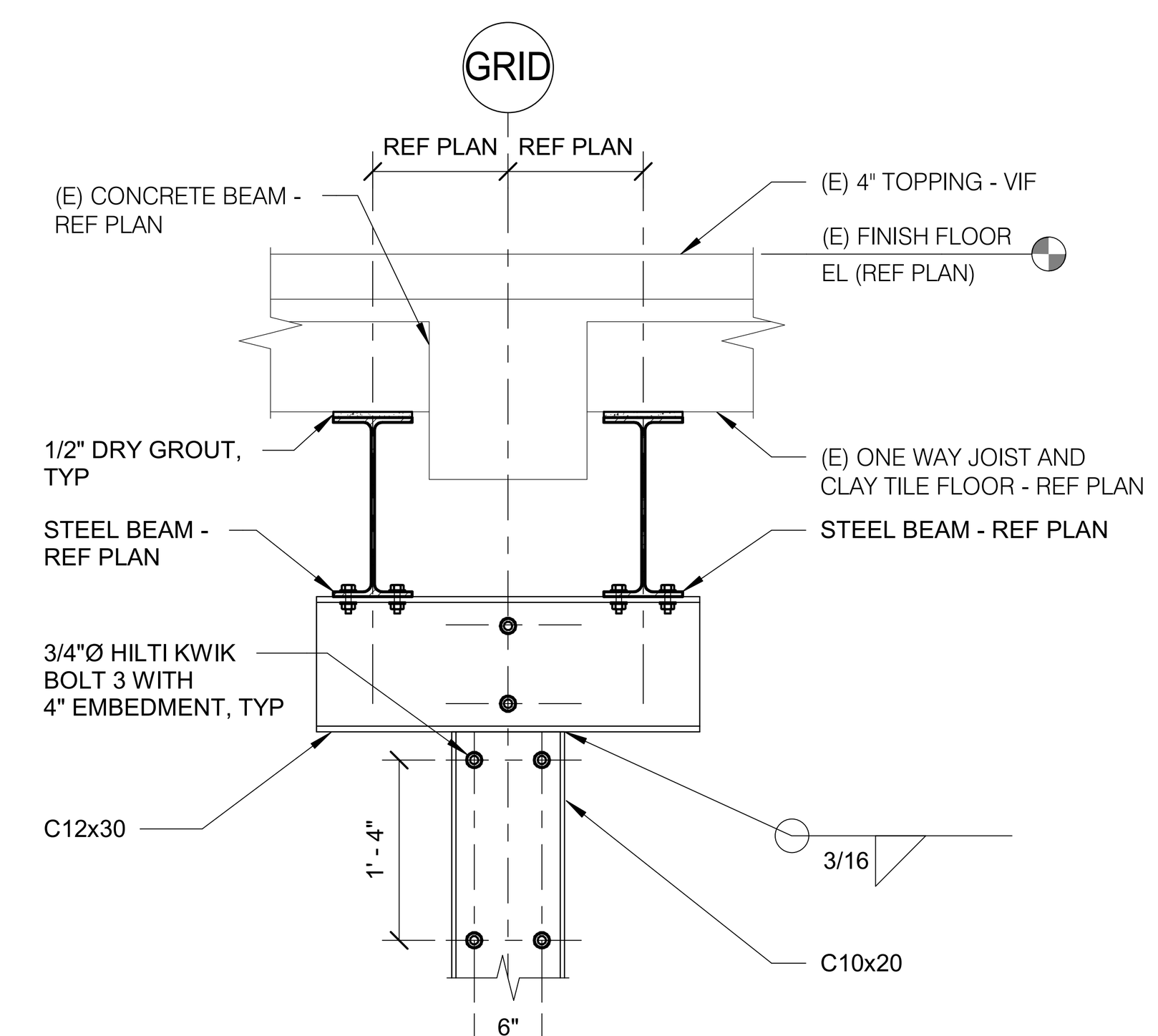
10 FRAMING AT SCREEN DETAIL

3/4" = 1'-0"



11 CONDENSING UNIT SUPPORT FRAMING

3/4" = 1'-0"

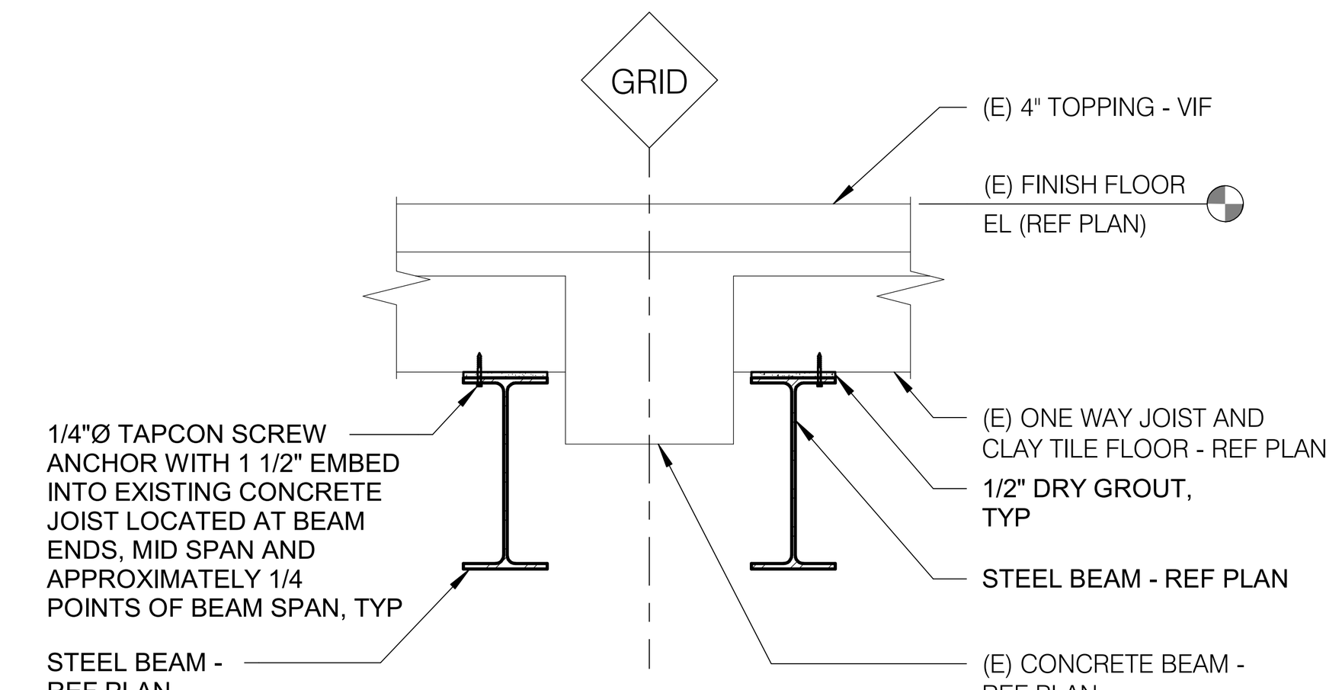


12 BEAM BEARING AT COLUMN

3/4" = 1'-0"

NOTES:

1. AT SIM: BOTTOM OF BEAM ELEVATIONS DIFFER. PROVIDE HSS2 1/2x2 1/2x3/16 SHIM AT BEAM BEARING OF HIGHER BEAM.



13 BEAM BRACING DETAIL

3/4" = 1'-0"

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Madison Municipal Building Renovation

BPW Project #7939
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Madison, WI 53703

I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the Laws of the State of Wisconsin.
ARCHITECT SEAL

Signature:
Print Names:
Date: License No:

ISSUE

MARK	DATE	DESCRIPTION
	03.24.2017	BID SET

PROJECT NO. 2014057

PROJECT PHASE BID SET

DRAWN BY: PRIPAN CHECKED BY: ABBPER

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www.kjww.com
PROJECT # 00140628.00

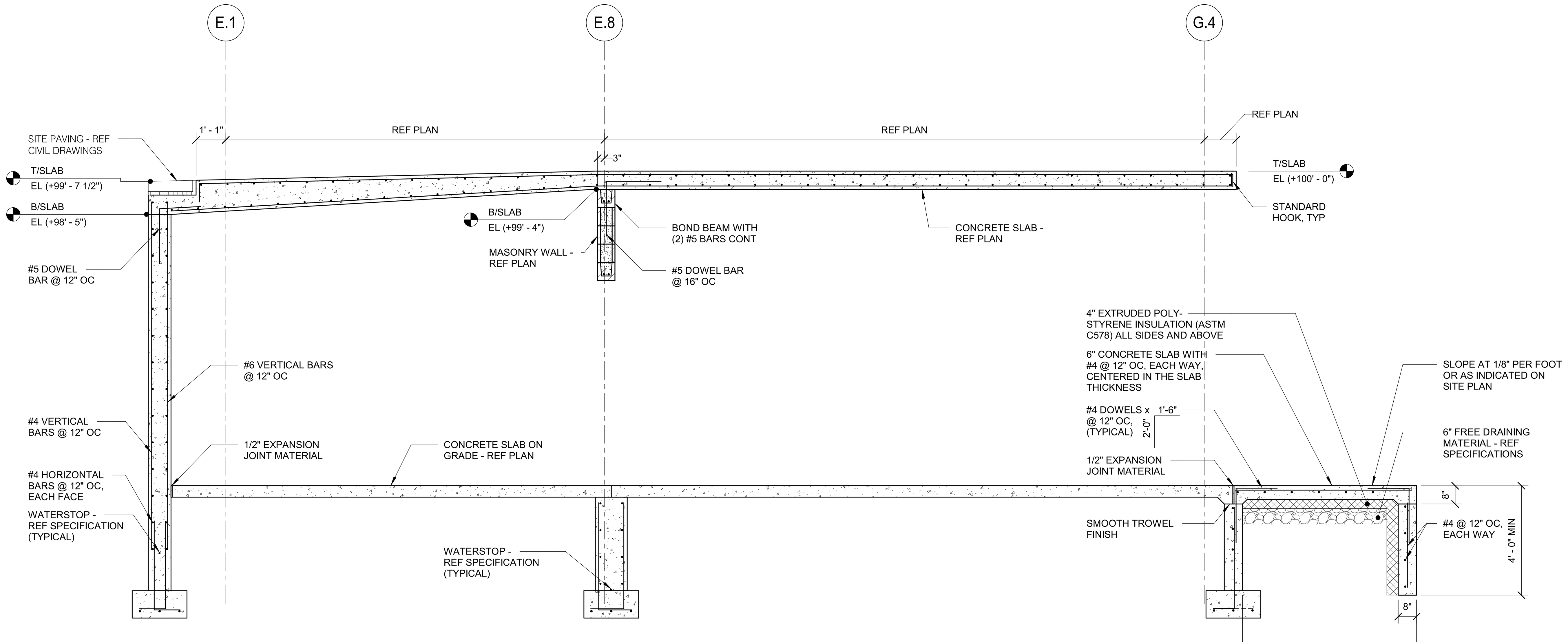
REFERENCE SCALE IN INCHES
0 1 2 3

FRAMING DETAILS

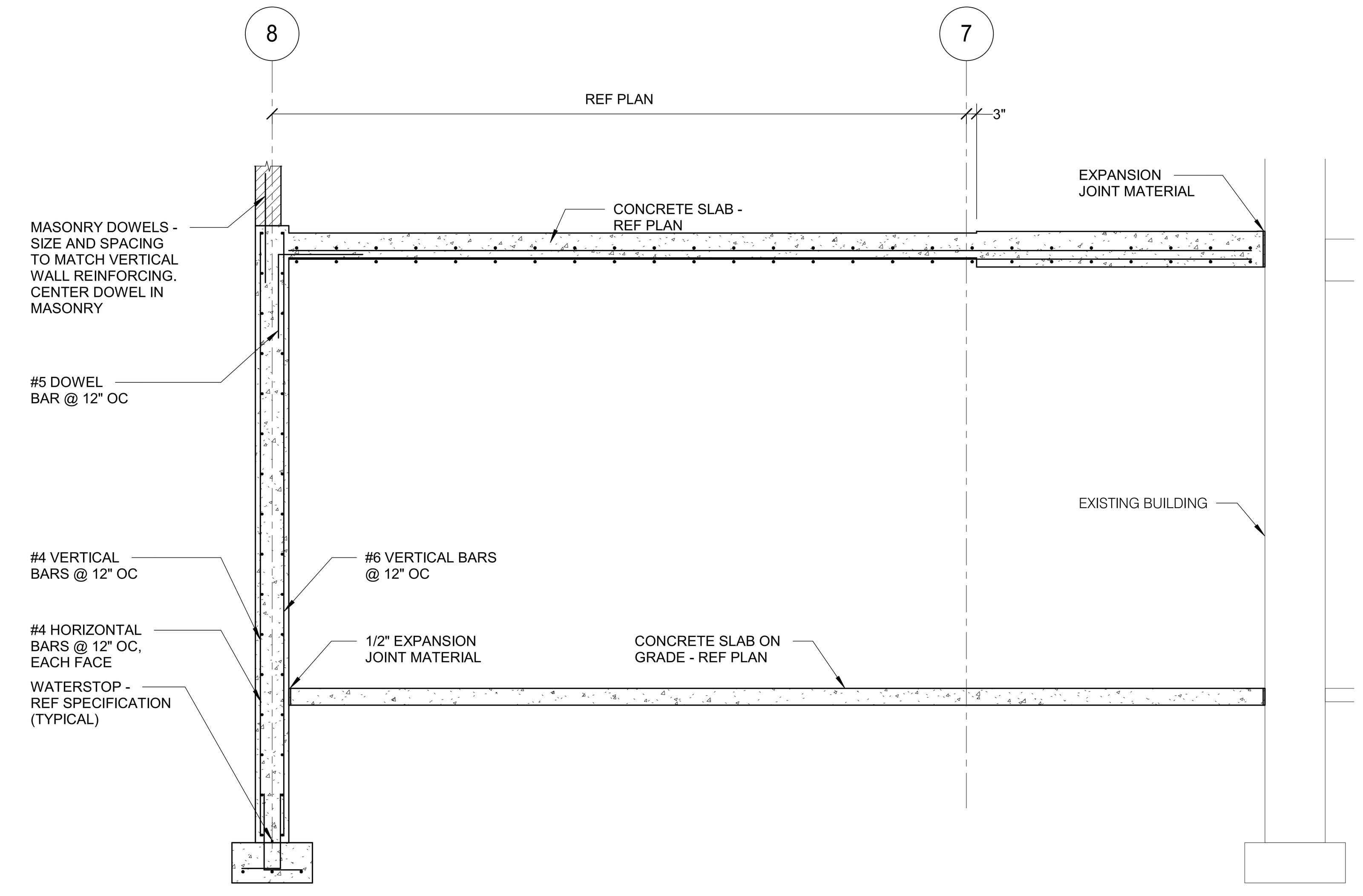
EXHIBIT H

S302

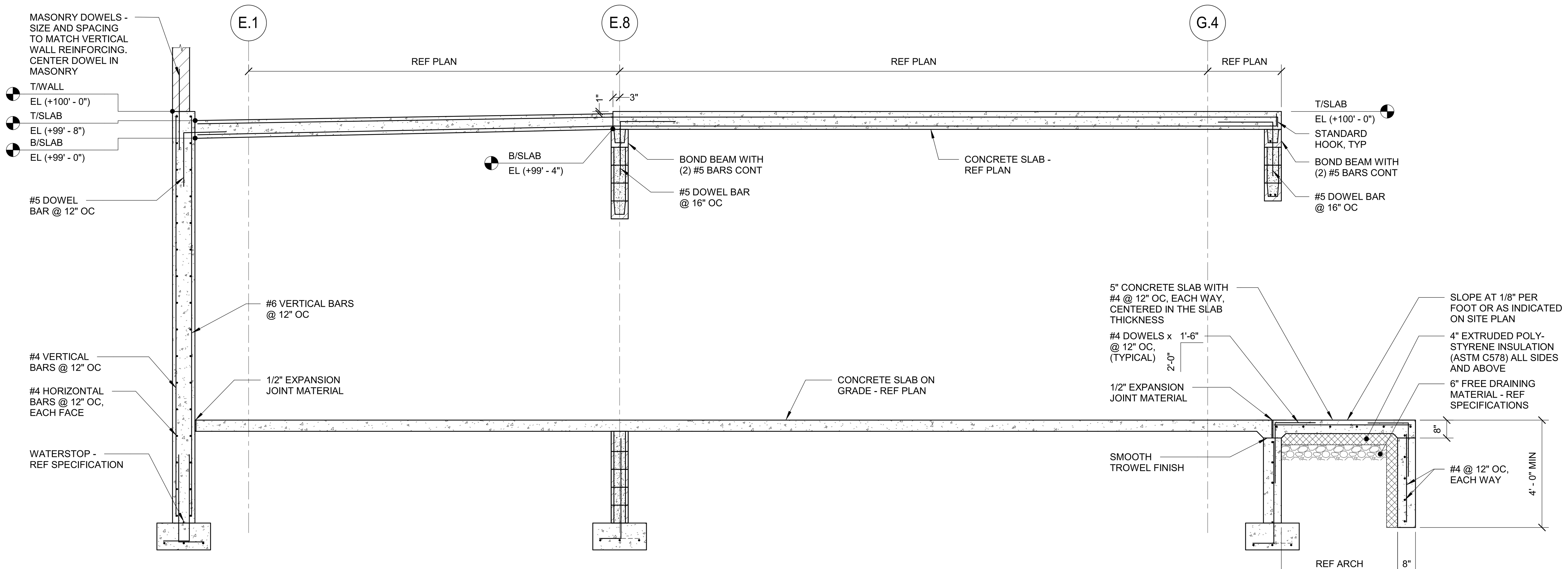
NOT FOR CONSTRUCTION



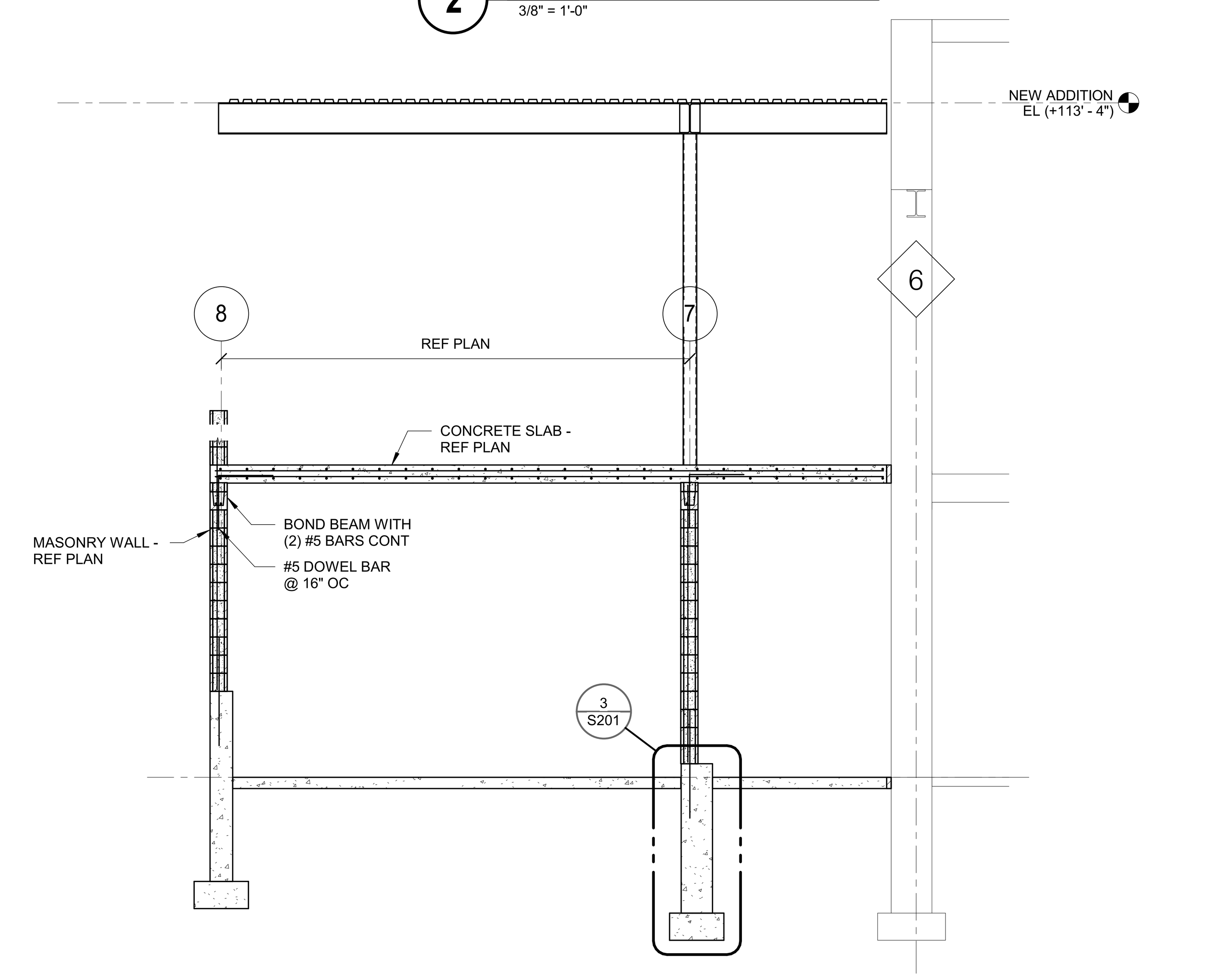
1 **BUILDING SECTION**
3/8" = 1'-0"



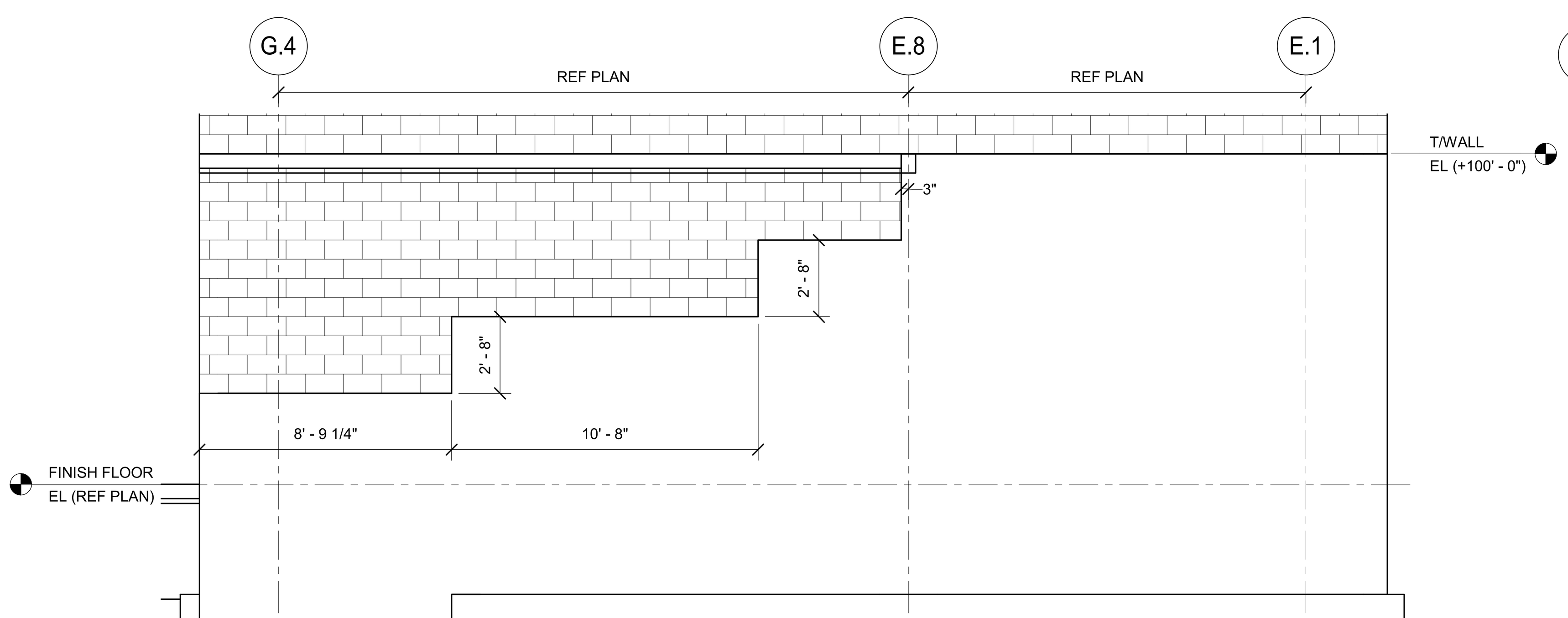
2 **BUILDING SECTION**
3/8" = 1'-0"



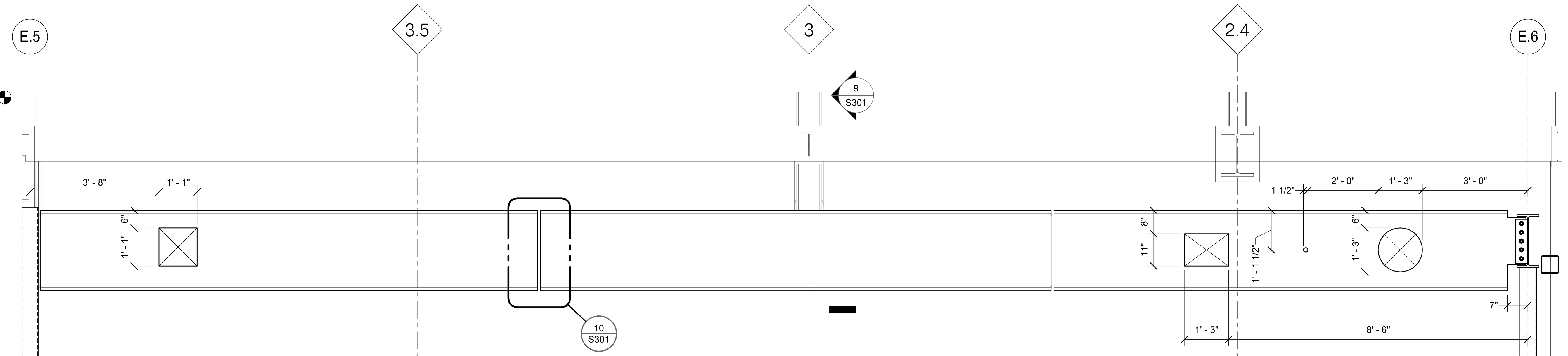
3 **BUILDING SECTION**
3/8" = 1'-0"



4 **BUILDING SECTION**
1/4" = 1'-0"



5 **BUILDING SECTION**
1/4" = 1'-0"



6 **BEAM ELEVATION**
1/2" = 1'-0"

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ARCHITECT SEAL

Signature:
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ISSUE

MARK	DATE	DESCRIPTION
	03.24.2017	BID SET

PROJECT NO. 2014057
PROJECT PHASE BID SET
DRAWN BY: PRIPAN
CHECKED BY: ABBPER

ELEVATION

EXHIBIT H

S303

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PROJECT # 00140628.00

REFERENCE SCALE IN INCHES

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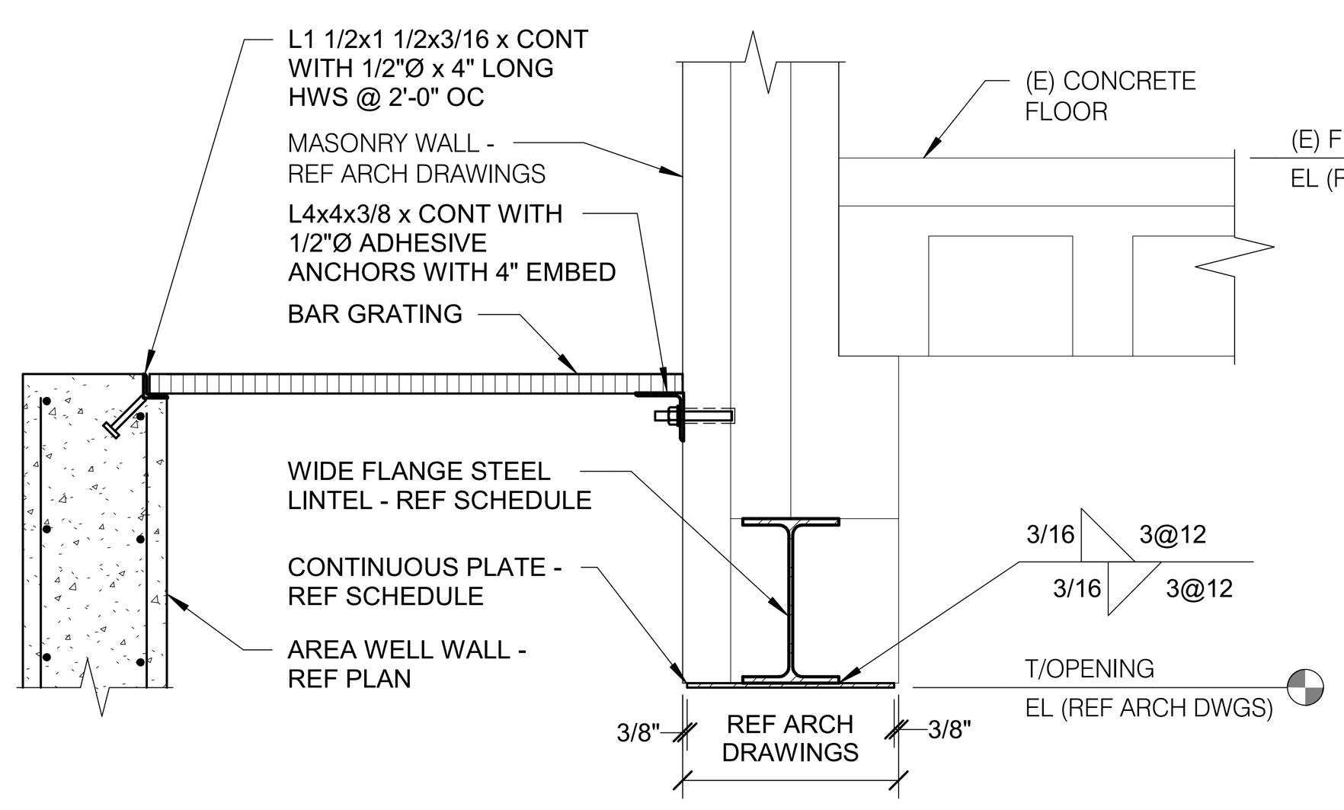
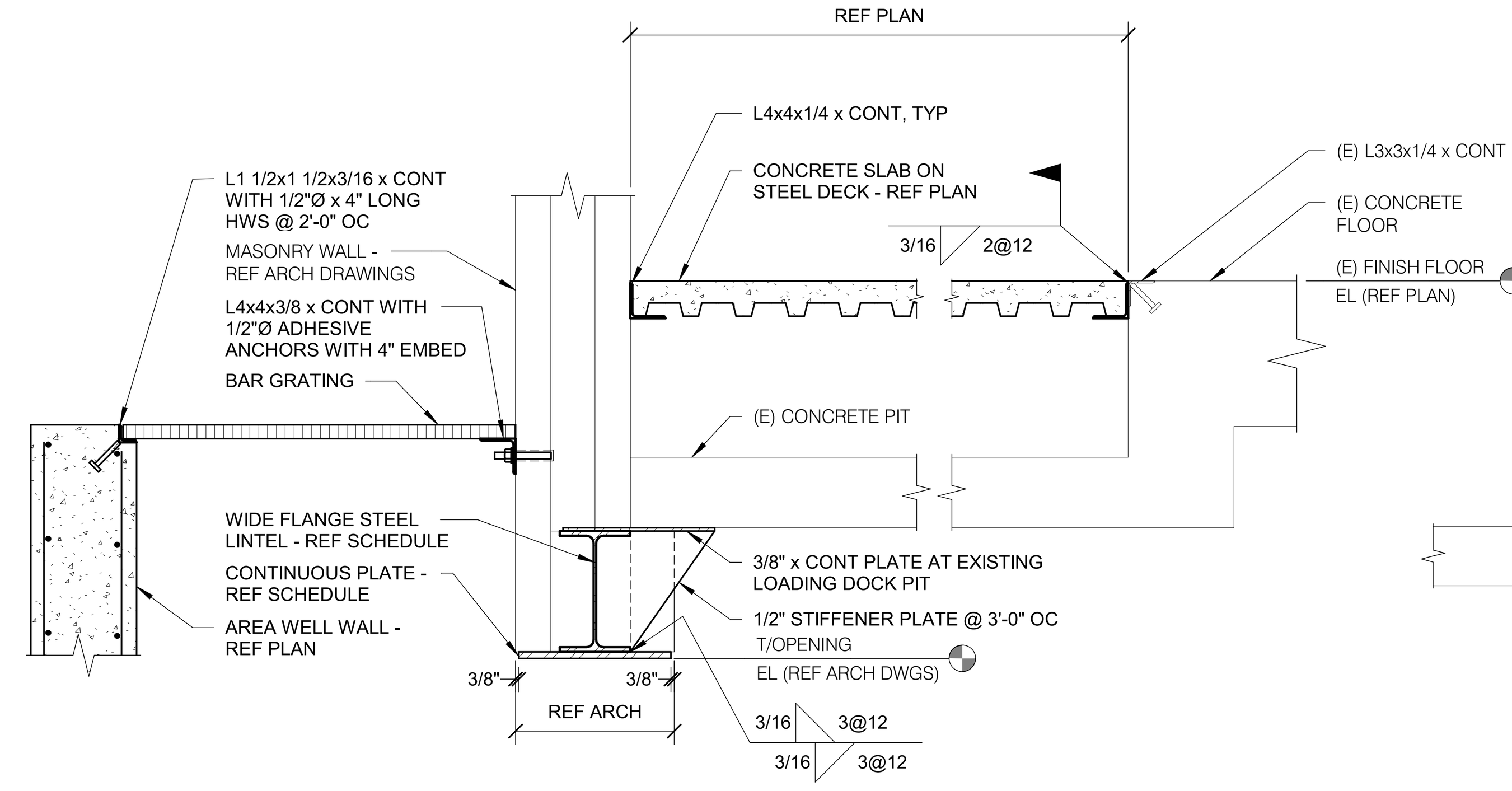
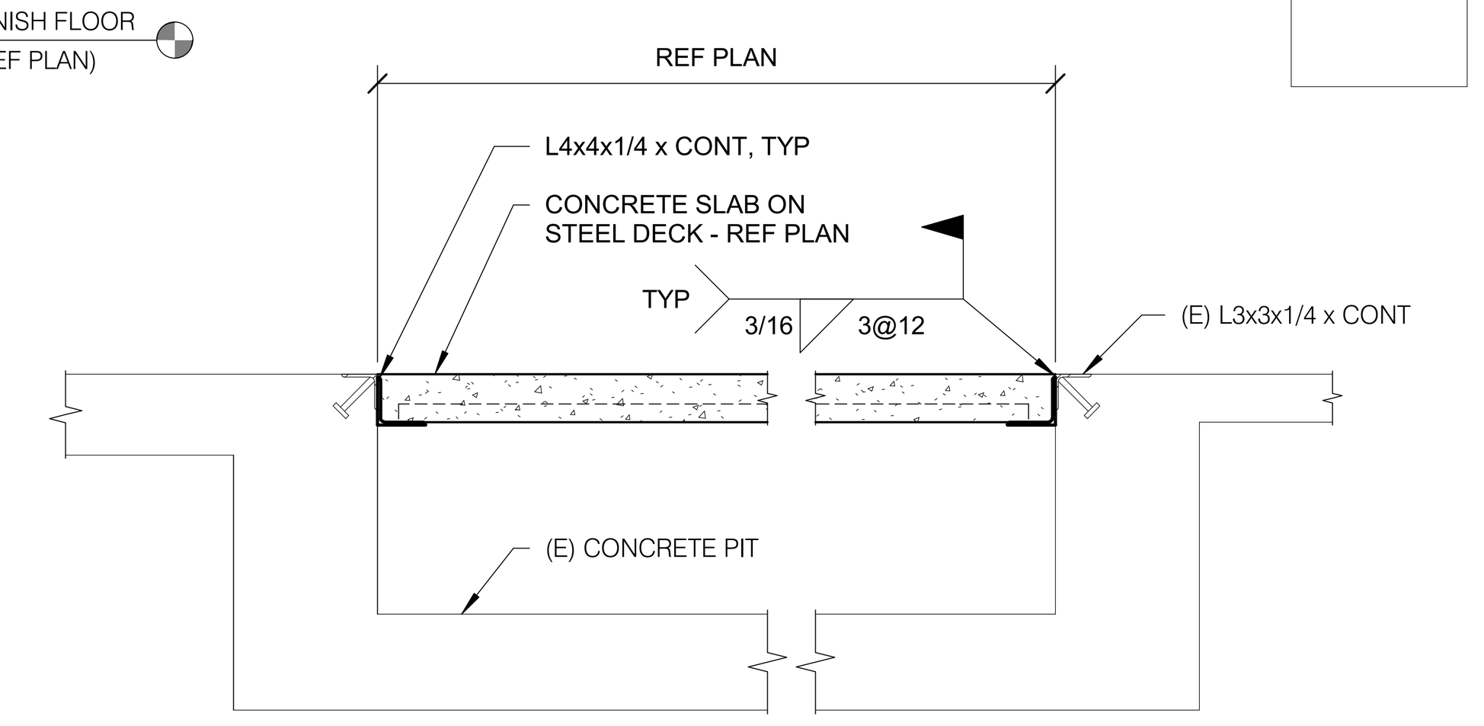


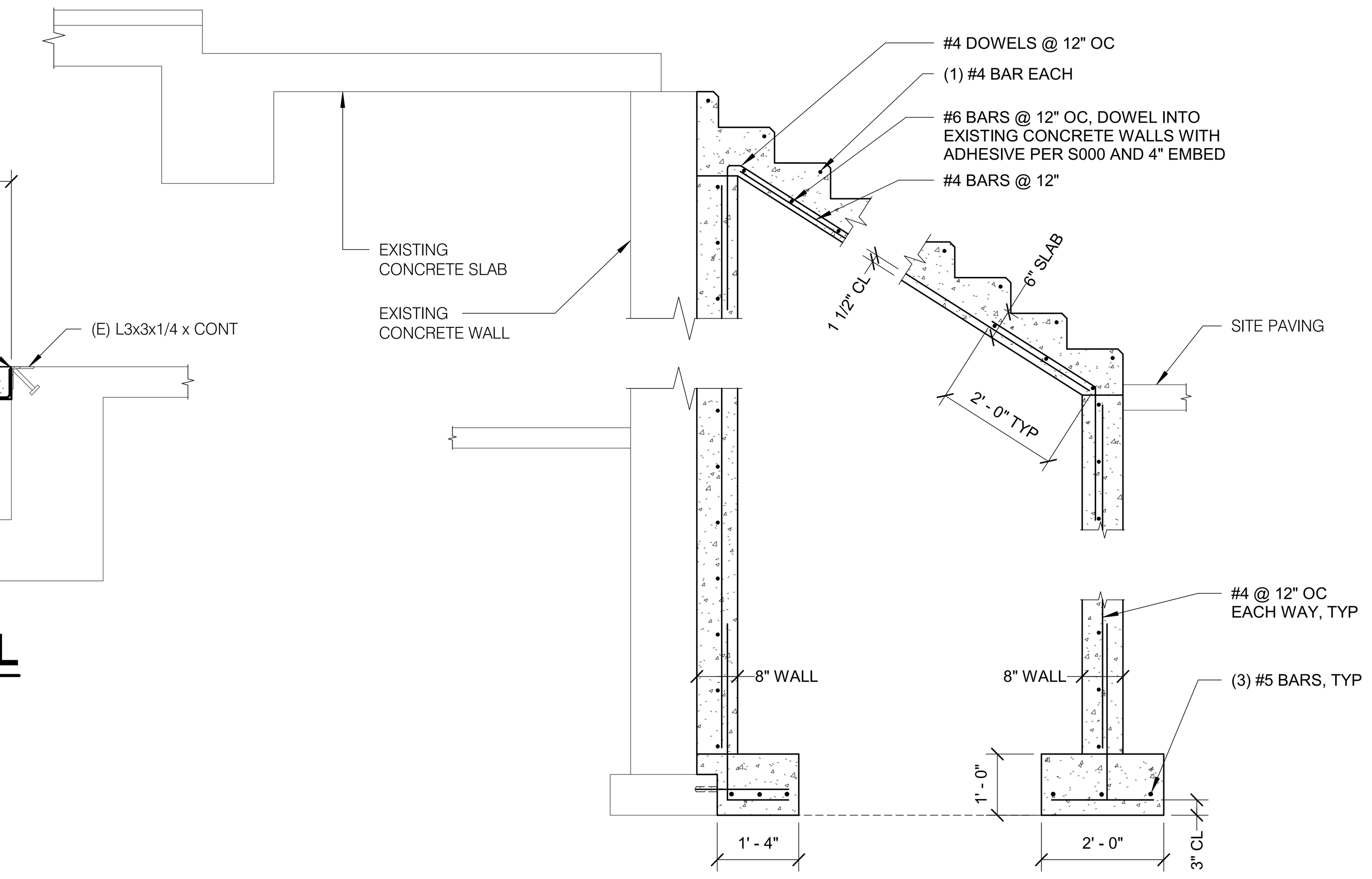
1 LINTEL DETAIL
3/4" = 1'-0"



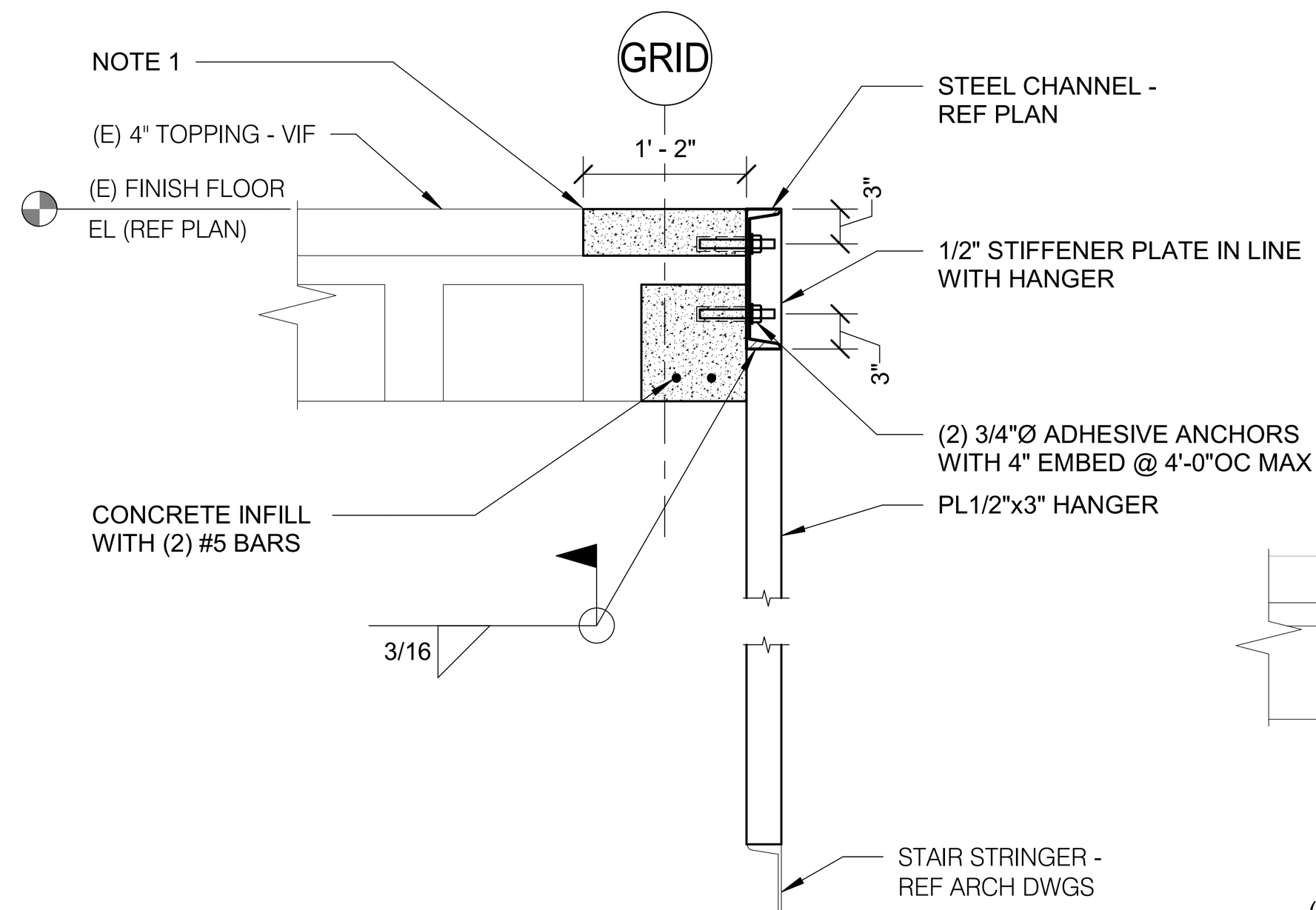
2 EXISTING LOADING DOCK SUPPORT
3/4" = 1'-0"



3 PIT INFILL DETAIL
3/4" = 1'-0"



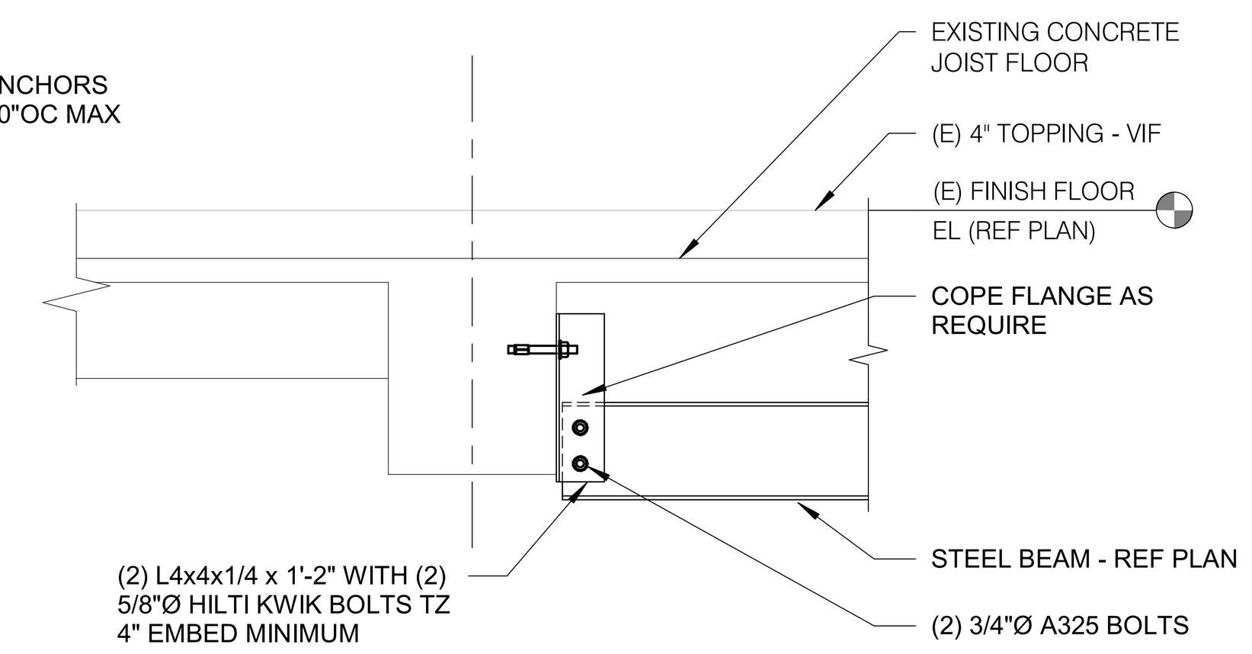
4 STAIR SECTION
1/2" = 1'-0"



16 STAIR SUPPORT DETAIL
3/4" = 1'-0"

NOTES:

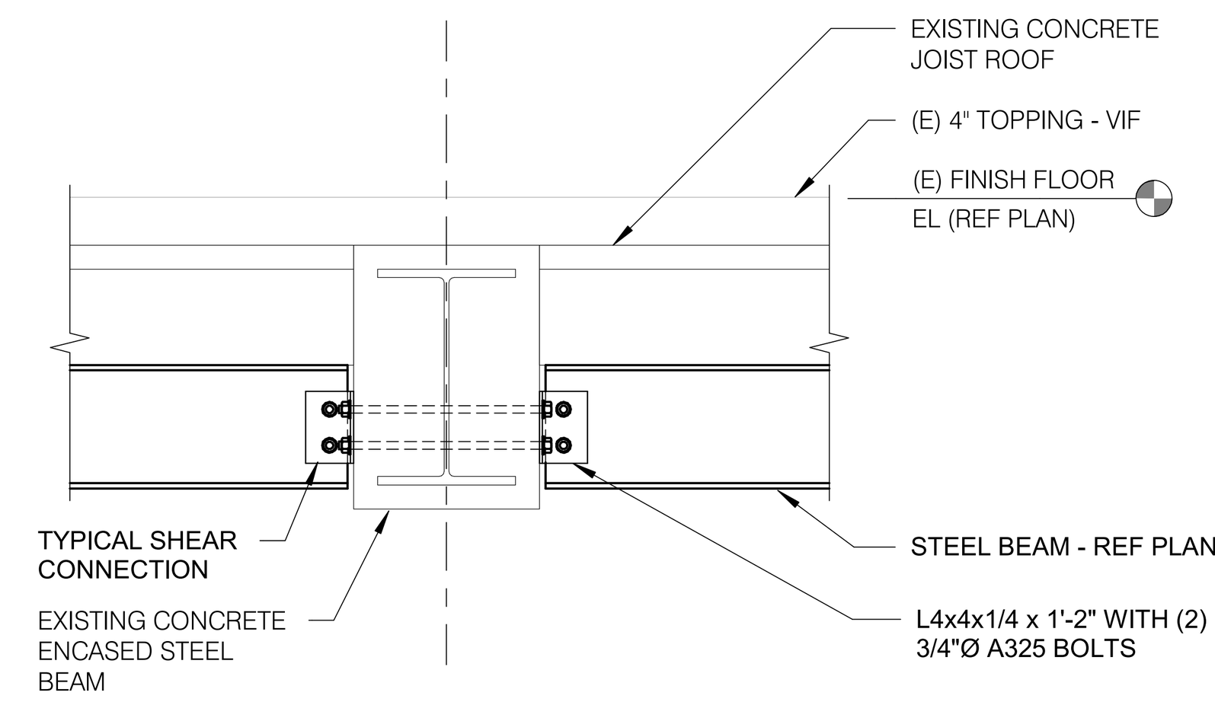
1. CONCRETE INFILL: DEMO EXISTING FLOOR FINISH AND POUR NEW CONCRETE INFILL. ROUGHEN SURFACE OF EXISTING CONCRETE BEAM. SURFACE MUST BE CLEAN AND DAMP AT TIME OF CONCRETE INSTALLATION.



6 FRAMING DETAIL AT EXISTING
3/4" = 1'-0"

NOTES:

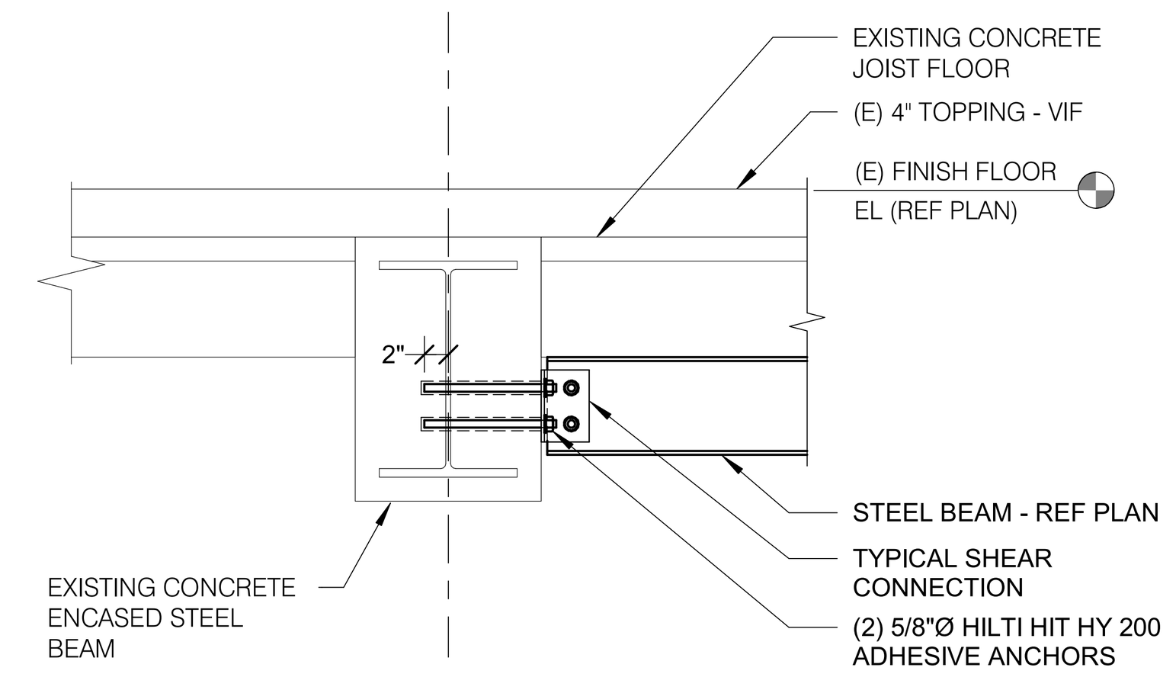
1. DEMOLISH EXISTING TILES AS REQUIRED IN FIELD TO PROVIDE NEW CONNECTION.



7 FRAMING DETAIL AT EXISTING
3/4" = 1'-0"

NOTES:

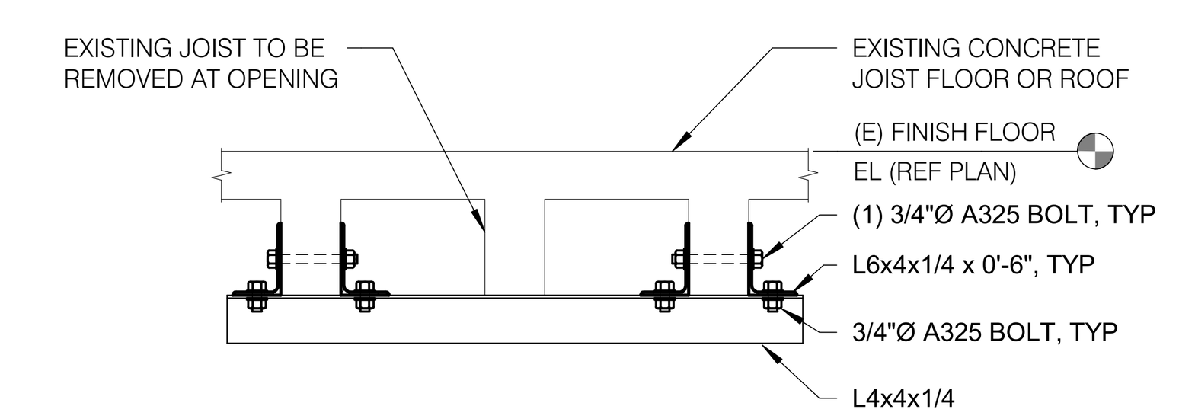
1. CONTRACTOR OPTION. REMOVE EXISTING CONCRETE AS NECESSARY TO WELD NEW PLATE WITH BEAM WEB. AND PROVIDE SHEAR CONNECTION PER 1/S302. AFTER COMPLETING WITH CONNECTION WORK POUR CONCRETE AS REQUIRE.



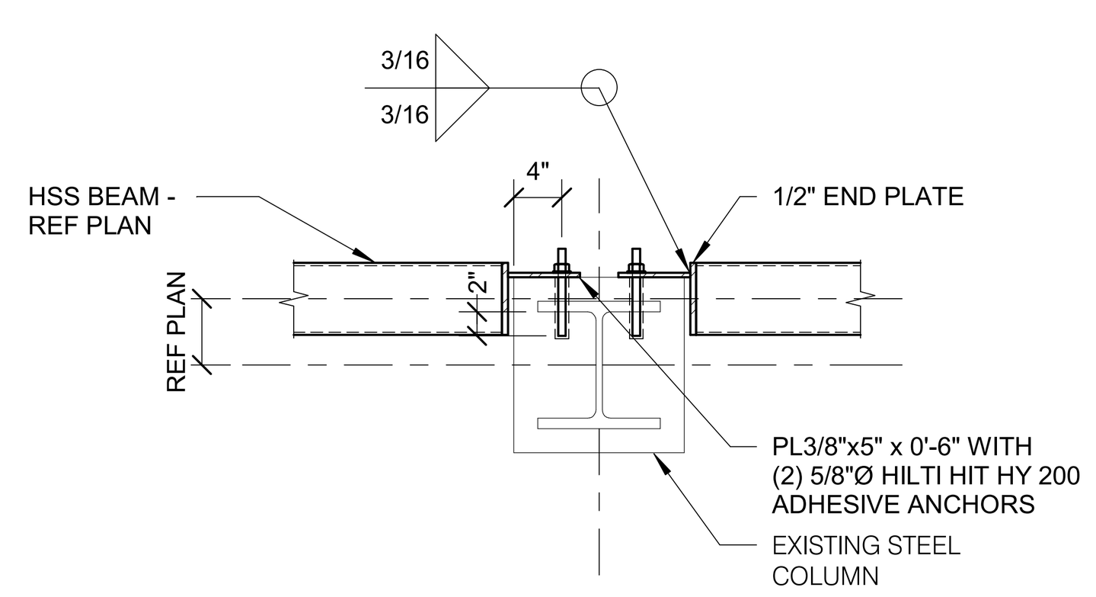
8 FRAMING DETAIL
3/4" = 1'-0"

NOTES:

1. CONTRACTOR OPTION TO REMOVE EXISTING CONCRETE FIREPROOFING AS NECESSARY TO FIELD WELD NEW SHEAR CONNECTION TO EXISTING BEAM WEB, SIMILAR TO 1/S302. FIREPROOFING MATERIAL TO BE REPLACED ONCE CONNECTION IS COMPLETE.



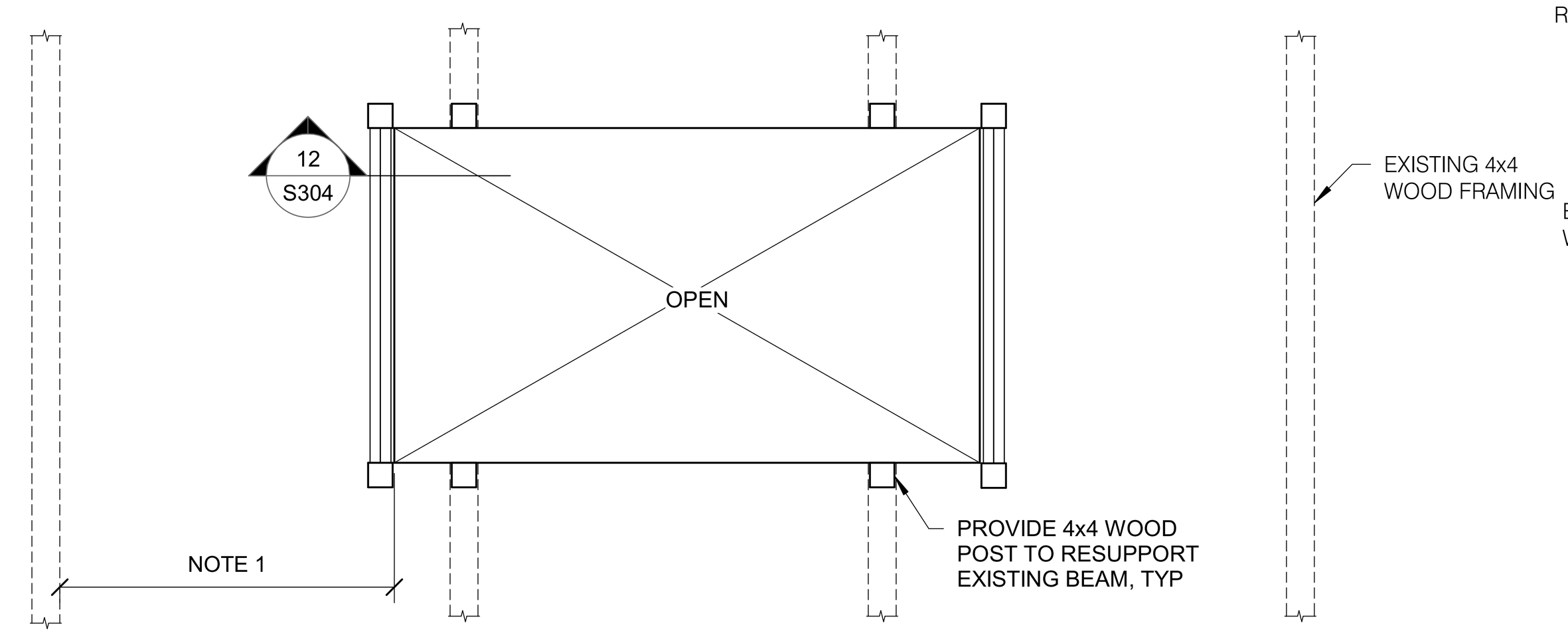
9 FRAMING DETAIL
3/4" = 1'-0"



10 FRAMING DETAIL
3/4" = 1'-0"

NOTES:

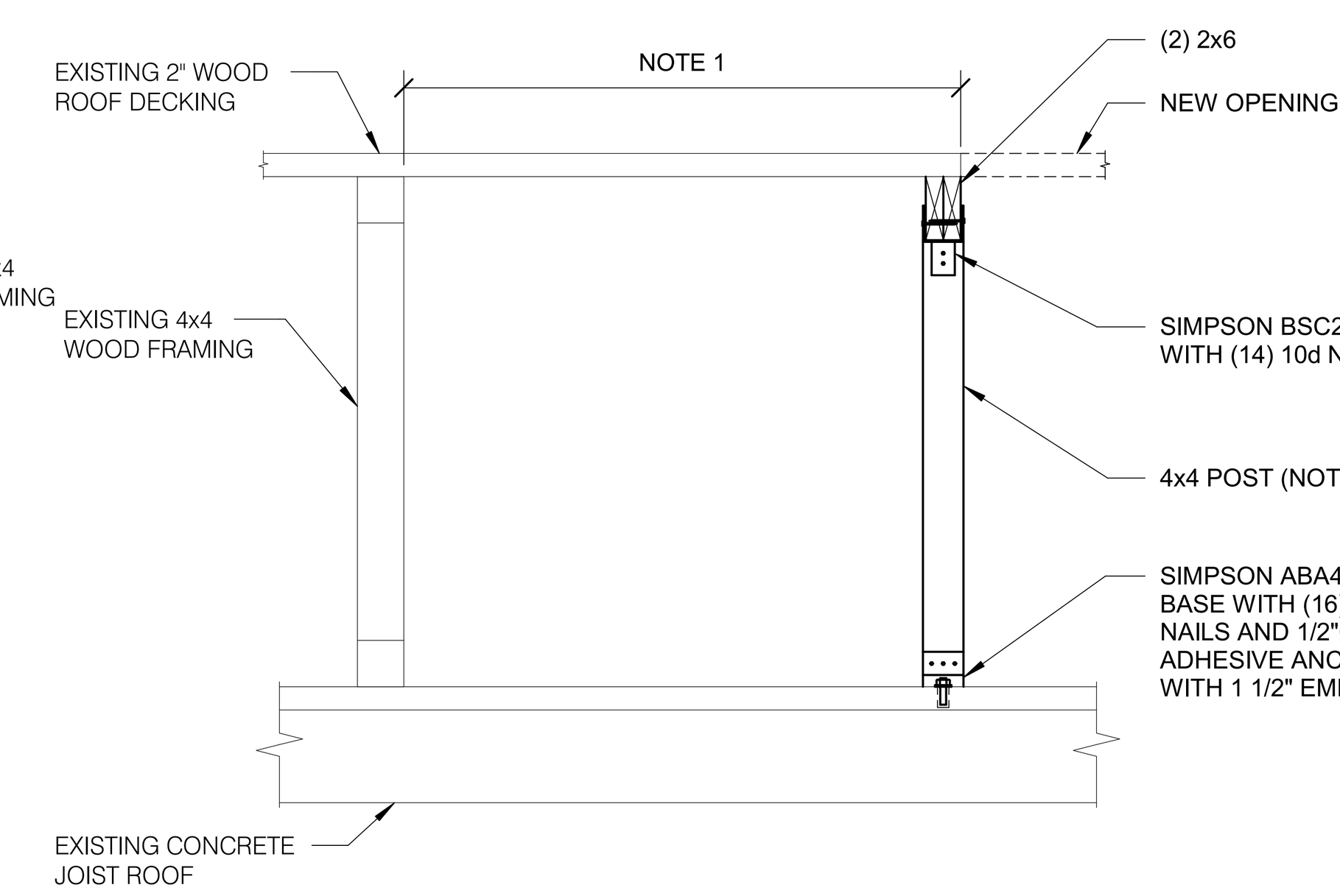
1. CONTRACTOR OPTION TO REMOVE EXISTING CONCRETE FIREPROOFING AS NECESSARY TO FIELD WELD NEW SHEAR CONNECTION TO EXISTING BEAM WEB, SIMILAR TO 1/S302. FIREPROOFING MATERIAL TO BE REPLACED ONCE CONNECTION IS COMPLETE.



11 ROOF OPENING DETAIL
1/2" = 1'-0"

NOTES:

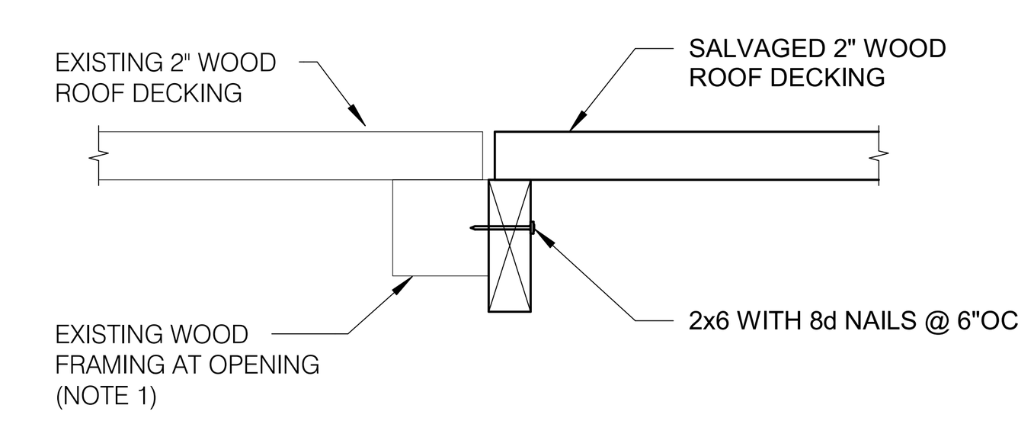
1. PROVIDE (2) 2x6 BEAM AND 4x4 POSTS WHEN DISTANCE TO EXISTING ROOF BEAM EXCEEDS 1' - 0".



12 DECK OPENING DETAIL
3/4" = 1'-0"

NOTES:

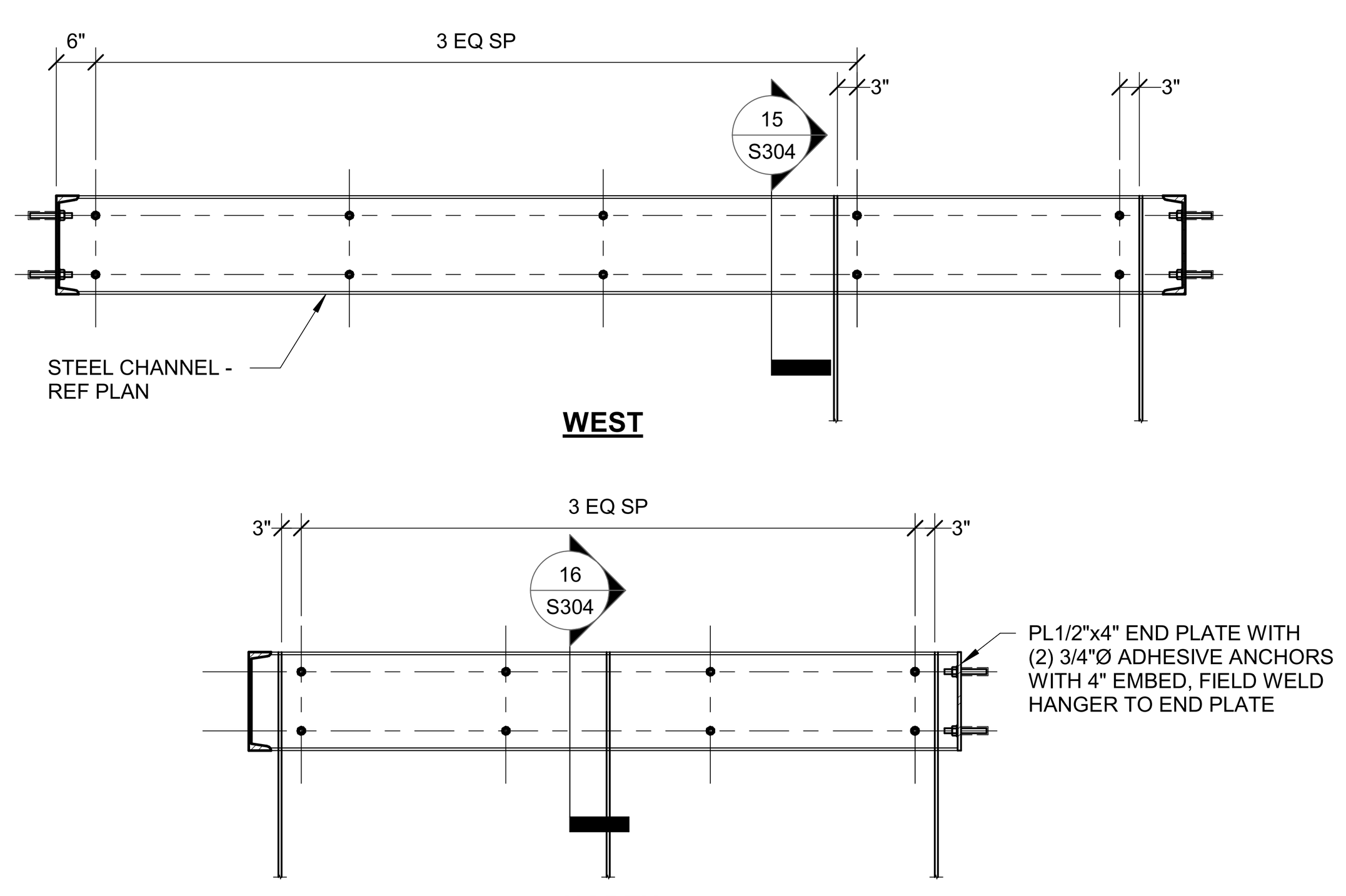
1. PROVIDE DECKING SUPPORT FRAMING WHEN DISTANCE TO EXISTING BEAM EXCEEDS 1' - 0".
2. EXISTING WOOD ROOF IS SLOPED. FIELD VERIFY POST HEIGHT.



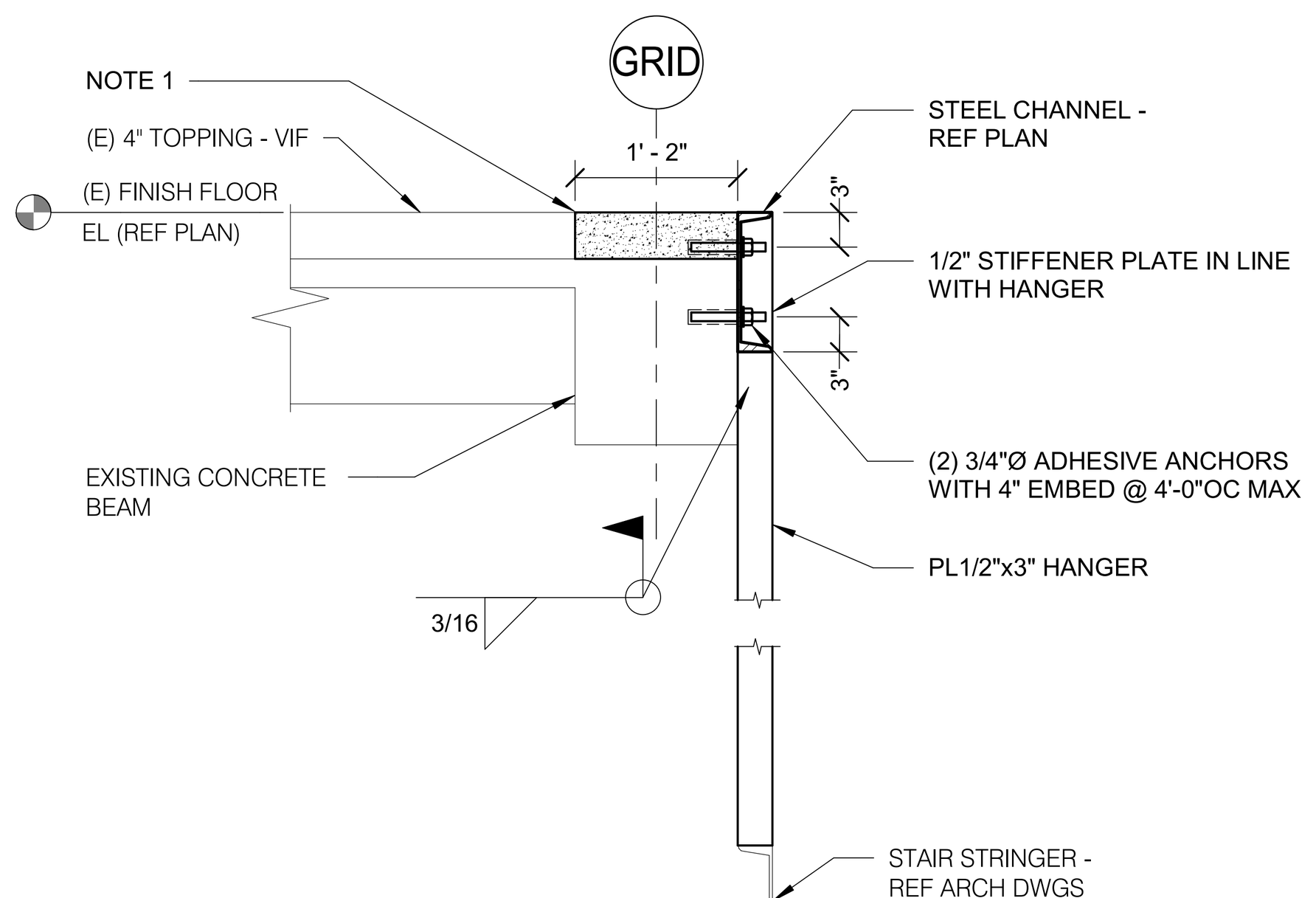
13 EXISTING ROOF PATCH
1 1/2" = 1'-0"

NOTES:

1. FIELD VERIFY EXISTING FRAMING AT OPENING. NOTIFY A/E IF EXISTING CONDITION DIFFER FROM WHAT IS SHOWN.



14 CHANNEL ELEVATIONS
1/2" = 1'-0"



15 STAIR SUPPORT DETAIL
3/4" = 1'-0"

NOTES:

1. CONCRETE INFILL: DEMO EXISTING FLOOR FINISH AND POUR NEW CONCRETE INFILL. ROUGHEN SURFACE OF EXISTING CONCRETE BEAM. SURFACE MUST BE CLEAN AND DAMP AT TIME OF CONCRETE INSTALLATION.

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I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the Laws of the State of Wisconsin.
ARCHITECT SEAL

Signature: ______
Print Names: ______
Date: ______ License No: ______

ISSUE

MARK	DATE	DESCRIPTION
	03.24.2017	BID SET

PROJECT NO. 2014057
PROJECT PHASE BID SET
DRAWN BY: PRIPAN CHECKED BY: ABBPER

FRAMING DETAILS

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REFERENCE SCALE IN INCHES
0 1 2 3

EXHIBIT H

S304

NOT FOR CONSTRUCTION